

Estimates for the Unsafe Return of  
Human Excreta to the Environment

Unsafe return of human  
excreta to the environment:  
a literature review

June 2015

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The Water Institute at UNC  
Gillings School of Global Public Health  
The University of North Carolina at Chapel Hill  
Rosenau Hall, CB #7431  
135 Dauer Drive, Chapel Hill, NC 27599-7431  
Phone +1-919-966-7302  
<http://www.waterinstitute.unc.edu>

**Authors:**

Ashley R. Williams and Alycia Overbo

**Reviewers and Editors:**

Pete Kolsky and Jamie Bartram

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Prepared by Ashley Rhoderick Williams and  
Alycia Overbo

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The Water Institute at UNC provides international academic leadership at the nexus of water, health and development.

Through **research**, we tackle knowledge gaps that impede effective action on important WaSH and health issues. We respond to the information needs of our partners, act early on emerging issues, and proactively identify knowledge gaps. By developing local initiatives and international **teaching and learning** partnerships, we deliver innovative, relevant and highly-accessible training programs that will strengthen the next generation's capacity with the knowledge and experience to solve water and sanitation challenges. By identifying or developing, synthesizing and distributing relevant and up-to-date **information** on WaSH, we support effective policy making and decision-taking that protects health and improves human development worldwide, as well as predicting and helping to prevent emerging risks. Through **networking and developing partnerships**, we bring together individuals and institutions from diverse disciplines and sectors, enabling them to work together to solve the most critical global issues in water and health.

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## 1. Abbreviations, Acronyms and Names

BOD – Biological oxygen demand  
COD – Chemical oxygen demand  
CSO – Combined sewer overflow  
CFU – Colony-forming unit  
EPA – United States Environmental Protection Agency  
FAO – Food and Agriculture Organization of the United Nations  
FSM – Fecal sludge management  
FSTP – Fecal sludge treatment plant  
IDK – Indah Water Konsortium  
JMP – Joint Monitoring Programme  
MDG – Millennium Development Goals  
MPN – Most Probable Number  
NGO – Nongovernmental Organization  
NSSMP – National Sewerage and Septage Management Programme  
OD – Open Defecation  
ODF – Open Defecation Free  
OECD – Organisation for Economic Cooperation and Development  
OWTS – Onsite wastewater treatment system  
PDC – Pollutant discharge per capita  
SDG – Sustainable Development Goals  
SS – Suspended solids  
SSO – Sanitary sewer overflow  
UDDT – Urine diverting dry toilet  
ULB – Urban local body  
UNC – University of North Carolina at Chapel Hill  
WaSH – Water, Sanitation and Hygiene  
WATSAN – Water and Sanitation  
WHO – World Health Organization  
WSP – Wastewater stabilization pond  
WWTP – Wastewater treatment plant

## 2. Executive Summary

This literature review was funded by Bill & Melinda Gates Foundation as part of a proof of concept project titled “Estimates for the Unsafe Return of Human Excreta to the Environment”. The aim was to compile evidence on the pathways and extent of unsafe return of human excreta to the environment throughout the sanitation delivery chain. This document was submitted to Bill & Melinda Gates Foundation on June 4, 2015. The authors invite readers to provide feedback, suggestions, and questions that may be addressed and incorporated in a future version of the review.

Within monitoring and evaluation, sanitation facilities are often assumed to be safe if, by design, they create a barrier between humans and human excreta. However, human excreta may be released into the environment if the waste is not sufficiently contained on-site, if the waste is “leaked” into the environment through improper disposal or transport, or if the waste is insufficiently treated. Human waste contains pathogens that are harmful to health; thus leakage of human excreta into the household, community, and greater environment is a public health concern.

This review investigated leakage of excreta in the containment, disposal, transport, treatment, and disposal stages of the sanitation delivery chain for the following technologies: pit latrines, septic systems, and sewerage. The review focused on “leakage” of fecal sludge, liquid waste stream, wastewater, and pathogens contained in excreta.

The review findings indicate that while there are few studies on “leakage” of latrines and septic systems, studies report that many latrines and septic tank are not emptied and are sources of groundwater contamination. Unlined latrines, damaged facilities, and pits serving as septic tanks do not provide effective containment and can cause microbial contamination of the household premises. Findings from a few studies indicate that latrines are widely affected by storms, heavy rainfall, and floods; while no studies reported on effects of weather events on septic systems, vulnerability to floods and extreme weather events may also be expected. In addition, findings indicated the additional hazard of households resorting to open defecation when their latrines became damaged or unusable.

While several studies and reviews cited latrines and septic systems as potential or likely causes of groundwater contamination events, this was not universal across included studies; some studies reported no contamination effects from nearby facilities. The range of findings emphasize that the impact of latrines and septic systems on groundwater quality is dependent on soil type, distance between groundwater and pit or drain field, and hydrological conditions. Additionally, seasonal effects on well contamination in areas with a high density of latrines or septic systems were reported in several studies.

Household latrine and septic tank emptying behavior is not well understood or characterized in the literature. Included studies on household emptying behavior for latrines and septic tanks commonly

reported a large proportion of respondents had never emptied their on-site sanitation facility or did not know the last time they had emptied it. This appeared to vary across study settings; some studies cited a high percentage of respondents routinely emptied their on-site sanitation facility. However, emptying was self-reported in all studies and subject to recall bias.

Methods for pit latrine emptying also varied by study setting; mechanical emptying was more prevalent in some regions, whereas manual emptying, burying pits, pit diversion, and mechanical emptying were more commonly used in other study settings. “Flooding” latrines appears to be a common practice in certain areas, yet was cited in few papers. Literature findings indicate that availability of diverse emptying options was associated with routine emptying. Several studies cited that while private and public companies may provide emptying services, they were often not sufficient to meet regional demand. Only two studies in peri-urban and rural areas were found, therefore the behaviors practiced in these settings when pits fill up are relatively undocumented.

Very little literature was found on certain topics related to on-site sanitation facilities. Septic systems are used in many urban and rural settings of developing countries, yet few studies on septic system performance and maintenance were retrieved from developing countries. Within a small number of studies, septic system maintenance was found to be infrequent. Older septic systems are prone to failure and common in the US, but little data was found on system age and performance.

Critical gaps identified in the literature included the fate of collected fecal sludge, and the extent of primary, secondary, and tertiary treatment. While some studies reported volumes of fecal sludge collected, treated, and properly disposed in certain cities, there were no estimates or studies found for many regions. Having more reliable estimates from collection through disposal would better illustrate regional gaps and opportunities within the sanitation delivery chain. Similarly, there are global estimates for wastewater that is treated, but the effectiveness and level of treatment is unknown. The results from the reviewed studies show, even with advanced treatment processes, some wastewater effluent still contains high levels of pathogens.

In order to understand the increased hazard to public health through the unsafe return of human excreta, it is necessary to determine where excreta is “leaking” back into the environment. From the literature, it is unclear what fraction of sludge is being disposed of, untreated, into surface water through practices such as flooding or discharging sludge into drains which may lead to wastewater treatment plants or directly into surface water through storm water drains. Since the rate of pathogen die-off varies in soil and water, future research on fecal sludge management behavior should report more specifically the location of disposal in order to better characterize the associated public health risks.



### 3. Introduction

As part of the United Nations Millennium Development Goals (MDGs), Target 7c focuses on reducing by half, “the “proportion of the population without sustainable access to ...basic sanitation” between 1990 and 2015. In order to monitor the progress towards this target, the Joint Monitoring Programme (JMP) has defined categories of improved and unimproved sanitation facilities. Improved sanitation facilities are ventilated improved pit latrines (VIP), pit latrines with a slab, composting toilets, and flush or pour-flush to either piped sewer systems a septic tank or a pit latrine. Unimproved sanitation facilities are flush or pour-flush to an endpoint other than a piped system, septic tank or pit latrine, pit latrine without a slab, an open pit, bucket, hanging toilet or hanging latrine, any type of shared facility, or open defecation (1).

One of the motivations behind this target is to reduce the risk of human exposure to hazardous pathogens present in human excreta by creating a barrier between humans and excreta. Previous systematic reviews have shown sanitation technologies to reduce the prevalence of diarrheal disease and helminth infections (2–4). While global monitoring currently focuses solely on the type of sanitation technology used by the household, there is a need to understand what happens with human excreta beyond the point of containment. The entire sanitation delivery chain (containment, emptying, transport, treatment, and disposal/reuse) must be examined in order to ensure a separation of human contact from human excreta within and beyond the household premises (5).

For each sanitation technology, there exist potential pathways for unsafe leakage of human excreta back into the environment. Pit latrines and septic tanks are considered “on-site sanitation” technologies and have some similar characteristics with regards to the sanitation delivery chain. Both technologies are designed to contain fecal waste at the household. Pit latrines generally contain fecal sludge along with some inorganic materials that are occasionally discharged into the pit by users. Septic tanks generate both a liquid and solid (sludge) waste stream, however the liquid waste is designed to discharge into the surrounding soil through a drain field. On-site sanitation technologies are used by roughly a quarter of Americans (6), and are the predominant sanitation technology in certain parts of Asia and Africa (7) (Table 1).

**Table 1. Extent of on-site sanitation usage in various regions.**

County	City/Region	% using on-site sanitation	Reference
Burkina Faso	Ouagadougou	88%	(8)
Bangladesh	Dhaka	79%	(9)
	Khulna	98%	
	Faridpur	99%	
Ghana		75%	(10)
	Kumasi	86%	(11)
Jamaica		70%	(12)
Nepal	Kathmandu Valley	30%	(13)
Senegal	Dakar	60%	(14)
		65%	(15)
U.S.		25%	(6)

While functioning pit latrines and septic systems provide a barrier between human excreta and users, structural failures and flooding may result in excreta being leaked into the ground water. According to JMP, in 2010 an estimated 30% of households worldwide used boreholes and dug wells for their primary drinking water source (16). Globally, the percentage of households relying on groundwater is even greater, since groundwater is the source for many piped systems and public taps. A review on factors affecting groundwater contamination in the U.S. and Canada found that within 55 included studies, over 60% of groundwater contamination reports were linked to septic and sewage systems (17). Graham and Polizzotto (18) conducted a recent review of the impact of pit latrines on groundwater quality and found that bacteria and viruses could travel up to 25 m and 50 m from pit latrines, respectively. Human excreta leaked from pit latrines and septic systems into groundwater may affect not only proximal households with wells or boreholes, but downstream users as well.

In addition to potential groundwater contamination, emptying pit latrines and septic tanks may introduce waste and pathogens into the environment. Once pit latrines and septic tanks reach their volumetric capacity, the accumulated fecal sludge must be emptied. Users may choose to empty the sludge or to simply bury the pit or tank (19); however, in densely populated urban areas, burying latrines becomes less of an option (20). The removal of fecal sludge from pit latrines and septic tanks can be performed manually or mechanically using vacuum tankers (20,21). Manual emptying places workers at risk to hazardous pathogens including *Ascaris*, *Trichuris* and *Taenia* (22), and can often result in human excreta being tracked into the household area. During the emptying process, it is unclear what fraction of sludge may spill within the household premises. Once the sludge is removed from the latrine or tank, it may be transported to a treatment facility or tipped at an official dumpsite, or potentially discharged into open fields, ditches, or waterways (21,23,24).

Specific fecal sludge treatment plants (FSTP) are not always available and in some cases, sludge is co-treated with other wastewater streams. The efficiency of different treatment processes varies and

can result in potentially hazardous “treated” fecal sludge being discharged back into the environment.

For households served by sewerage connections, transport, treatment and disposal are of particular concern for potential leakage. During transport within sewers, hazardous return of unsafe excreta can occur due to misconnections, structural deficiencies, and flooding events such as combined sewer overflows (25–27). However, a previous study has highlighted that a significant pathway of “unsafe return” for sewage is likely within the treatment step. Baum et al. (28) estimated 1.5 billion people have sewer connections that do not lead to any type of treatment facility but discharge sewage back into the environment.

There are multiple pathways in the sanitation chain that may introduce pathogenic waste into households, communities and the environment, and a more thorough understanding of the leakage of human excreta is needed to safeguard public health. This literature review was conducted in order to further examine and identify “leakage” pathways along the sanitation delivery chain.

## 4. Methods

Targeted Boolean searches were conducted in Web of Science and Google Scholar between March 15 and April 24, 2015. Search strategies included terms for latrines, septic systems, sewerage, and wastewater treatment. Additional search terms were used for the stages in the sanitation delivery chain, such as emptying, transport, and treatment efficiencies.

Papers from peer-reviewed journals and grey literature were included in this review. Eligible grey literature included papers from conference proceedings and reports. Accepted papers had qualitative or quantitative findings on sanitation technology functionality, microbial contamination, emptying, transport, treatment, or groundwater contamination. Bibliographies from accepted papers were also searched for relevant papers.

Data on study findings were extracted from included papers. Findings from accepted papers were grouped by sanitation technology and by phase in the sanitation delivery chain.

## 5. Results

### 5.1 Microbial hazard of fecal waste streams

One of the primary hazards associated with human fecal waste is the presence of pathogenic organisms in feces. Tables 2 and 3 show the average concentrations of various pathogens present in human excreta, wastewater, and fecal sludge.

**Table 2. Average number of pathogens in fresh feces.**

	<b>Pathogen</b>	<b>Fresh feces Mean organism/g wet feces</b>
Bacteria	Bacteroides	$10^{7.3-11}$
	Bifidobacterium	$10^{8.5-10.0}$
	Campylobacter jejuni	$10^7$
	Citrobacter	$10^8$
	Clostridia <sup>1</sup>	$10^{4.7-10}$
	Enterobacteria	$10^{6.7-9.4}$
	Enterococci	$10^{5.3-8.1}$
	Eubacteria	$10^{8.5-10}$
	Pathogenic <i>E. coli</i> <sup>1</sup>	$10^8$
	Fusobacterium	$10^9$
	Klebsiella	$10^8$
	<i>Lactobacilli</i>	$10^{4.9.0}$
	Peptostreptococcus	$10^{10}$
	Proteus	$10^8$
	<i>Ruminococcus</i>	$10^{10}$
	<i>Salmonella spp</i>	$10^{6-8}$
	<i>Shigella spp</i> <sup>1</sup>	$10^{6-7}$
<i>Vibrio cholerae</i> <sup>1</sup>	$10^{6-7}$	
<i>Yersinia enterocolitica</i>	$10^5$	
Viruses	Enteroviruses	$10^{6-7}$
	Rotavirus	$10^6$
	Hepatitis A	$10^6$
Protozoa	<i>Entamoeba histolytica</i>	$10^5$
	<i>Giardia lamblia</i>	$10^5$
Helminths	<i>Ascaris lumbricoides</i> <sup>1</sup>	$10^4$
	<i>Clonorchis sinensis</i>	$10^2$
	<i>Diphyllobothrium latum</i>	$10^4$
	<i>Fasciolopsis buski</i>	$10^3$
	Helminth eggs <sup>a</sup> (eggs/L) (Strauss)	20,000-60,000
	Hookworms <sup>a</sup>	$10^2$
	<i>Schistosoma mansoni</i> <sup>a</sup>	40
	<i>Strongyloides stercoralis</i>	10
	<i>Taenia saginata</i>	$10^4$
<i>Trichuris trichiura</i> <sup>a</sup>	$10^3$	

Sources: (29–32)

<sup>a</sup>The distribution of these pathogens in excreta is highly dependent upon the prevalence within the community.

**Table 3. Concentrations of pathogens present in wastewater and fecal sludge by region.** Adapted from (33,34).

Pathogen	Country/Region	Wastewater	Fecal sludge <sup>a</sup>	
Fecal coliforms (CFU/100 mL)	Ghana	10 <sup>4</sup> -10 <sup>9</sup>		
	Mexico	10 <sup>7</sup> -10 <sup>9</sup>		
	USA	10 <sup>6</sup> -10 <sup>9</sup>		
Salmonella spp. (CFU/100 mL)	Mexico	10 <sup>6</sup> -10 <sup>9</sup>		
	USA	10 <sup>3</sup> -10 <sup>6</sup>		
Protozoan cysts (per L)	Mexico	978-1814 <sup>b</sup>		
	USA	28 <sup>c</sup>		
Helminth eggs (per L)	Developing countries	70-3000	70-735	
	Brazil	166-202	75	
	Egypt	N/A		67 (Mean)
				735 (Maximum)
	Ghana	0-15		76
				4,000-25,000
	Jordan	300		N/A
	Mexico	6-98		73-177
	Morocco	214-840		N/A
	Pakistan	142 <sup>d</sup>		N/A
	Philippines			5,700
	Thailand			4,000
	Ukraine	20-60		N/A
	France	9-10		5-7
	Germany	N/A		<1
Great Britain	N/A		<6	
Irkutsk, Russia	19		N/A	
USA	1-8		2-13	

<sup>a</sup>Sludge from either latrines or septic tanks

<sup>b</sup>*Entamoeba histolytica*, *Giardia lamblia* and *Balantidium coli*

<sup>c</sup>*Cryptosporidium*

<sup>d</sup>Ascaris

Although the concentration of pathogens in feces varies in different regions according to the prevalence of infection within the region, Lucena et al. (35) observed similar concentrations of fecal coliforms, enterococci, spores of sulphite-reducing clostridia, somatic coliphages, F-specific RNA bacteriophages, and bacteriophages infecting *Bacteroides fragilis* in sewage from Europe, North America, and Latin America. Lucena et al. (35) did not include sewage samples from Asia and Africa and did not analyze concentrations of protozoa and helminthes, so regional variations in pathogen concentrations may still be expected.

The die-off rate of pathogens varies according to type, temperature, and media (e.g. surface water, groundwater, or soil). Table 4 gives the average and maximum survival times and/or time for 90% inactivation ( $T_{90}$ ) for different pathogens in sludge, soil, and freshwater or sewerage. Some studies have observed coliform survival up to five and a half months (29). The survival of bacteria and viruses are likely to be longer in groundwater than surface water (29), and die-off rates have been shown to

decrease with decreasing temperature (36). However, higher temperatures may reduce the survival of protozoa (37).

**Table 4. Survival times and die-off of excreted pathogens in various media.** From (29,38).

Pathogen	Feces, sludge 20-30°C Average days (Max)	T <sub>90</sub> faeces (days, mean ± SD)	Soil 20-30°C Average days (Max)	T <sub>90</sub> soil (days, mean ± SD)	Freshwater and sewage at 20- 30°C Average days (Max)
<b>Bacteria</b>					
Fecal coliforms	<50 (<90)		<20 (<70)		<30 (<60)
<i>Salmonella</i> spp	<30 (<60)		<20 (<70)		<30 (<60)
<i>Shigella</i> spp	<10 (<30)				<10 (<30)
<i>Vibrio cholera</i>	<5 (<30)		<10 (<20)		<10 (<30)
<i>Salmonella</i>		30 ± 8		35 ± 6	
EHEC <sup>a</sup>		20 ± 4		25 ± 6	
<b>Viruses</b>					
Enteroviruses	<20 (<100)		<20 (<100)		<50 (<120)
Rotavirus		60 ± 16		30 ± 8	
Hepatitis A virus		55 ± 18		75 ± 10	
<b>Protozoa</b>					
<i>Giardia</i>		27.5 ± 9		30 ± 4	
<i>Cryptosporidium</i>		70 ± 20		495 ± 182	
<b>Helminths</b>					
<i>Entamoeba histolytica</i> cysts	<15 (<30)		<10 (<20)		<15 (<30)
<i>Ascaris lumbricoides</i>	Many months	125 ± 30	Many months	625 ± 150	Many months

<sup>a</sup>EHEC – Enterohaemorrhagic *Escherichia coli*

The initial concentrations of pathogens in fecal waste and pathogen die-off rates determine the relative hazard of excreta over time. Excreta with low microbial concentrations or inactive pathogens may be less hazardous than excreta with higher pathogen concentrations or lower die-off rates. Thus, estimates of the fraction of human excreta “unsafely” returned to the environment would ideally consider the pathogens present in excreta as well as their initial concentrations and die-off over time.

## 5.2 Latrines

A recent systematic review estimated that 1.77 billion people worldwide use pit latrines as their primary sanitation facility (18). While there are numerous types of pit latrines, the JMP classifies pit latrines with a slab, ventilated pit latrines (VIP), and pour flush pit latrines as “improved sanitation”. Pour flush latrines that are not connected to a pit latrine, pit latrines without a slab, bucket latrines, hanging latrines, and open pits are all considered “unimproved sanitation” technologies. Published studies were reviewed to identify and quantify potential pathways of unsafe return of human excreta associated with improved and unimproved pit latrines.

### 5.2.1 Containment

At the containment level, compromised structural integrity, flooding, and leakage of waste into surface water and groundwater were identified as potential pathways of unsafe return.

#### *Structural integrity*

Lined pit latrines can reduce the risk of microbial leakage from excreta into groundwater (39). In a cross-sectional study of 662 households in Dar es Salaam, 88% of surveyed households had a traditional pit latrine, and less than a quarter (21%) of them were fully lined. The majority (41%) of traditional latrines were unlined. Including VIP, pour flush, and bucket latrines, 36% of latrines were unlined, 36% were partially lined and 23% were fully lined. Additional structural compromises such as a slightly cracked, badly cracked or collapsing slab were observed in 39.9%, 13.4% and 6.2% of latrine facilities (40). A study of 100 households in Malawi found 95% of latrines were unlined (41).

#### *Flooding*

Flooding poses another potential “leakage” of waste for pit latrines located in flood prone areas. In areas of severe flooding, the majority of sanitation facilities can become inundated with water (42). Flooding and severe weather can cause structural damage to latrines resulting in excreta being washed into peoples’ homes and into the streets (43,44). No specific study was identified that examined the impact of flooding on fecal contamination from sanitation facilities, however four studies reported on latrine flooding events or vulnerability. A cross-sectional study of 189 households in Nicaragua found 41% were located in flood zones and 37% of sampled households had a latrine overflow within the past year (45). In peri-urban Malawi, Grimason et al. (41) found 47% of surveyed households experienced pit collapse, with 23% citing heavy rainfall as the main contributing factor. A study of post-cyclone damage in affected areas of Bangladesh found 90% of latrines were damaged or destroyed in the wake of the event (46). A rapid assessment study in Vientiane, Lao PDR reported 31% of toilets (the majority were pour-flush latrines) were at risk of flooding, and 6% had flooded at least once (47). The same study also observed pit latrines filling with water due to high groundwater (47).

#### *Surface water contamination from pit latrines*

Two studies were reviewed that demonstrated microbial contamination of surface water bodies due to unsafe return of pit latrine waste. Latrines can be “flooded out” by creating a trapdoor or pipe in the bottom of the latrine, allowing sludge to run off with rainwater (48,49). A study of protected springs in Kampala, Uganda cited effluent from flooded out pit latrines as a potential source of fecal coliform contamination (48). The study found statistically significant differences in fecal coliform concentration in protected springs between high population density and low population density villages. The springs served as a primary drinking water source for lower-income households in the area, thus the households were at risk of exposure to waterborne pathogens (48).

A study in Bangladesh also examined the impacts of latrines on the microbial quality of nearby ponds (50). In the study area, 43 ponds were sampled, of which 11 were used for fishing or bathing, 16 had no specific purpose and 16 were identified as “latrine ponds” since they directly received latrine effluent. Results from PCR analysis of human and bovine Bacteroidales revealed human fecal contamination was more prevalent than bovine fecal contamination. Latrine ponds had the highest

concentrations of fecal indicator bacteria (FIB) (25<sup>th</sup> and 75<sup>th</sup> percentiles being  $7.9 \times 10^3$ ,  $2.2 \times 10^5$  MPN/100mL). While ponds used for bathing and fishing had significantly lower concentrations of FIB (25<sup>th</sup> and 75<sup>th</sup> percentiles being  $5.1 \times 10^2$ ,  $4.4 \times 10^3$  MPN/100mL), the concentrations were still higher than the U.S. Environmental Protection Agency's recreational water limit of 126 MPN/100mL (50).

### **Groundwater contamination from pit latrines**

A recent review of the impact of pit latrines on groundwater quality included 20 studies that measured microbiological contamination of groundwater (18). Nineteen studies measured fecal indicator bacteria concentration (total coliforms, fecal coliforms, fecal streptococci, *E. coli*, or *B. coli*), while one measured rotaviruses and adenoviruses (51). None of the studies examined protozoa or helminth contamination, although these have been shown to have relatively small movement in groundwater (52).

Of the 20 studies included in the review, 17 reported microbial contamination associated with pit latrines. Travel distance varied from 1-25 m for bacterial contamination, although the authors remarked that most reported transport distances were closer to half the distance of the maximum value (~10 m). In the one study examining viruses, viral contamination of water sources was significantly associated with at least one latrine within 50 m (51). Soil type and hydrological conditions influenced the extent of microbial contamination and travel distance of pathogens.

Lined pit latrines and increased vertical separation between pit latrines and ground water tables were recommended to decrease groundwater contamination (39,53–55). In areas where the water table is high, households may construct raised pit latrines to increase vertical separation (48,56,57). Chaggu et al. (56) observed 50% of latrines were full due to high groundwater. However, one study reported that elevated pit latrines can actually lead to a greater risk to groundwater water contamination since it may increase the hydraulic gradient between groundwater (58).

In contrast, four studies found no significant correlation between pit latrine density and poor groundwater microbial quality (59–62). Overall, attributing groundwater contamination to pit latrines may be difficult since microbial pathogens from agriculture, livestock, and solid waste may leach into groundwater from surface infiltration and runoff. A 1999 WELL Report on groundwater and latrines concluded the key factor to be considered is the residence time between the point of contamination and the point of water withdraw (63). Depending on hydrological conditions and soil type, the residence time may be sufficient to allow microbial contaminants to die-off before abstraction.

### **Non-functionality of sanitation facilities**

Another pathway for unsafe return of excreta occurs when sanitation facilities are compromised and households must resort to open defecation. Several studies reported encountering sanitation facilities that were unusable due to flooding, structural collapse, or being full (40,42,64). In one study, latrines were non-operational for an average of 52 and 22 days due to a full pit or a collapsed pit, respectively (40). During periods of non-functionality, households must find an alternative sanitation option. Shimi et al. (42) found 48% of households resorted to open defecation during a flood event. The alternative may be temporary or become a permanent behavior change if

households chose not to empty their pit or repair significant structural damage. Hoque et al. (44) found 11% of households stopped using their latrines after a flood, while a study in Ghana found 63% of households preferred to practice open defecation once their pit was full (64).

### 5.2.2 Emptying

The frequency of pit emptying ranged from pits that had never been emptied to some that were emptied every month due to high groundwater and heavy rainfall (65). The required frequency for pit emptying depends on the pit depth, volume, and accumulation rate, which is dependent on multiple factors including the number of users, amount of excreta generated per person, drainage factors, and the volume of inorganic material discarded into the latrine (20,66). Table 5 shows average pit emptying frequencies reported in literature.

**Table 5. Average years between emptying and average fill rates for pit latrines.**

Country	Technology	Years between emptying	Filling rate (years)	Reference
Ghana	Pit latrine		4.2 (3 months-10 yrs)	(67)
Ghana	Pit latrine		6-10	(64)
Ghana	Pit latrine		4.2	(11)
Kenya	Pit latrines	0.8		(23)
Tanzania	Pit latrines	0.1		(65)
Tanzania	Pit latrine (unlined)	8.2		(49)
	Pit latrine (partially lined)	6.5		
	Pit latrine (fully lined)	8.5		
	Pit latrine (drum/tire)	4.7		
	Septic/sewer	5.5		
Tanzania and Uganda	Pit latrines	0.5		(68)

A Water and Sanitation Program field note reported 13% of pit latrines in the Kibera slum were full at the time of the survey (23). In a cross-sectional study of pit latrine emptying behavior in Dar es Salaam, Jenkins et al. (40) found 10% of pits were completely full and 35% had 25 cm to the top of the slab. Only 5% of pits had more than 1 m between the sludge height and slab. In Ghana, field observations from a study of 270 households found 31% of pit latrines were full (64).

Regarding emptying behavior in Dar es Salaam, Jenkins et al. (40) found over half of the respondents (63.5%) had never emptied their latrine. Only 60% of households planned to empty their pit latrine once it became full, while 15% planned to replace it, and 25% were not sure what they were going to do (49). In a different study in Dar es Salaam, 73% of households desludged their tank when full, while 23% planned to build a new latrine and 5% did not know what to do (56). Appiah-Effah et al. (64) reported only 1.9% of 270 surveyed household desludged their household toilet, however a little over a quarter (26.3%) of surveyed households had a private latrine. Thus, the majority of surveyed households used shared facilities (69.6%) and may not have been aware of the emptying practices of the communal facilities (64).

Five main methods of handling full pits were identified through the literature search: mechanical emptying, manual emptying, pit diversion, 'flooding', or burial. Table 6 summarizes the distribution of reported emptying methods employed by households. Mechanical emptying utilizes a vacuum tanker that uses hoses to pump sludge out of the pits (20,21). Manual emptying can be done using portable manual pumping technology (such as manual pit emptying technology (MAPET) or gulpers) or it can be performed by hand using buckets and rakes. Mechanical and MAPET are considered hygienic emptying methods, while hand emptying, pit diversion and flooding are considered unhygienic (49). However, even when employing mechanical emptying methods, fecal sludge can leak into the household due to the poor conditions of the hoses and equipment (69).

While some households with sufficient land area may choose to cover a full pit over and construct a new latrine, others may choose not to construct a new latrine after burying a full pit. These households may opt for alternative sanitation options such as using shared sanitation facilities or reverting to open defecation (64). Observations from one study in Tanzania revealed 28% of latrines practiced flooding out their latrine, although only 12% of households admitted to practicing this method of disposal (40). This emptying method was observed but not quantified in two studies of households in Kampala, Uganda (48,57).

**Table 6. Summary of pit emptying practices reported in literature.**

Country	Region	Setting	Households total (N)	Households practicing emptying	Pit latrine emptying	Reference
Burkina Faso	Ouagadougou	U	1.5 million	50%	75% vacuum tanker	(70) as cited in (8)
Ghana	Ashanti region	P & R	270	1.9% (5)	1.9% mechanical emptying	(64)
Kenya	Kibera	U			80% flooded out 20% buried	(71)
Kenya	Kibera	U	49		33% mechanical emptying 28% manual emptying 13% gravitational emptying 3% chemical emptying 5% burial 13% out of order latrines 5% unknown	(23)
Lao PDR	Vientiane	U	548		99% mechanically <1% manual emptying	(47)
Malawi	Blantyre	U	100		82% buried pits, construct new 13% mechanical emptying	(41)
Mali	Bamako		306	70% (214)	80% Vacuum trucks 17% manual emptying 1% emptied by household 1% other (not specified)	(72)
Senegal	Dakar	U			74% discharged into streets 7% discharged onto compounds	(73) as cited in (74)
Senegal	Dakar, Thiès, Touba	U	1,500	64% (960)	64% mechanical emptying 26% manual emptying 9% both mechanical and manual emptying	(15)
South Africa	eThekweni Municipality	P & R	15,983	65.5% (10,414)	83.8% manual emptying (by household) 9.2% private company (unspecified)	(75)
Tanzania	Dar es Salaam	U	207	72% (149)	73% vacuum trucks 27% pit diversion	(56)

Tanzania	Dar es Salaam	U	662	36.5% (241)	59% pit diversion 18% vacuum tankers 12% flooded out latrines 5% Vacutug ~5% manual emptying	(49)
Tanzania	Dar es Salaam	U	379		60% manual emptying <sup>a</sup> 25% vacuum tanker or MAPET 5% discharge to drains	(68)
Uganda	Kampala	U	250		56% manual emptying 20% mechanical emptying 2% discharge to drains	

U= Urban, P= Peri-Urban, R= Rural

<sup>a</sup>Manual emptying methods included MAPET.

### Emptying services

The type of emptying service providers were usually private companies, government owned operators, or informal operators. Table 7 summarizes the number and types of formal service providers reported in the reviewed studies.

**Table 7. Reported formal emptying service providers from reviewed studies.**

Country	City/Region	Type of service	Number available	Reference
Bangladesh	Dhaka	NGO	2	(9)
	Faridpur	Public	1	
	Khulna	Public	1	
Ghana	Accra	Private	26	(76)
Ghana	Kumasi	Private	17	(11)
		Public	5	
Lao, PDR	Vientiane	Private	17	(47)
Senegal	Dakar	Private	50 <sup>a</sup>	(15)
	Thiès		5 <sup>a</sup>	
	Touba		12 <sup>a</sup>	
Tanzania	Dar es Salaam	Private	28	(56)
		City-council	14	

<sup>a</sup>Average number of private businesses

Key barriers to hygienic pit emptying arose within the reviewed literature. Lack of awareness of services was mentioned in two studies. One found 74% of surveyed households in Blantyre, Malawai were not aware of hygienic pit emptying services within their community (41), and the other study in Dar es Salaam, Tanzania reported 95% of households knew about vacuum tanker services (49). Availability and access of emptying services were cited as major barriers for households to empty pits hygienically. In a study in Dar es Salaam only 43% and 24% of households had vacuum tanker or Vacutug services available in their community (49). The same study found the odds of households emptying their latrine using a hygienic form of pit emptying increased 23 times if services were available in their area and their plot was accessible. Vacuum tankers are often unable to access latrines located in densely populated urban areas with narrow roads (9,15,41). Vacuum tanker access is site-specific; 96% of on-site sanitation facilities were accessible by vacuum truck in a study from Vientiane, Lao PDR (47).

### Technological limitations

Manual emptying is sometimes preferred over vacuum tankers and mechanical pumping due to limitations in technology or access (as well as cost, see below). Households that used hygienic emptying methods such as vacuum tankers complained that they were not as efficient as manual hand emptying and often times left sludge in the pit (49,69,74,77). In certain communities there are not enough vacuum tankers to meet the demand, resulting in delays for household latrines to be emptied (74). Aging equipment and breakdowns were also cited as limitations to providing hygienic emptying services (21,69). Mechanical pumping can often hindered due to the presence of inorganic, non-degradable items in the pit (23), resulting in communities manually desludging large, shared pit latrine facilities (64).

## Cost

Another major barrier for households to access hygienic pit emptying services is the cost. Studies reported households often knew more hygienic methods existed, yet they chose manual hand emptying because it was cheaper (49,64,68). However, one study in Kibera, Kenya reported mechanical emptying was the cheapest method, but accessibility and technology were its limiting factors (23).

When households share latrines, the financial burden to empty the pit may be seen as the owner's sole responsibility rather than a shared expense by all users (57). However, the cost of emptying services were found to be too great for one household to afford in a Dar es Salaam study (49). In Tanzania and Uganda, Isunju et al. (68) and Katukiza et al. (57) observed household latrine construction and emptying was financed by landlords or owners. Isunju et al. (68) observed some landlords neglected to build or maintain latrines as they felt access to a public latrine was sufficient (68). One study examined emptying practices of shared dormitory sanitation facilities in Vientiane, Lao PDR (47). Eight dormitories had septic tanks and two had pour-flush pit latrines. Three of the facilities had never been emptied but reportedly had yet to fill completely. All seven facilities that were emptied used a vacuum truck service, and the frequency of emptying ranged from annually to once every three years (47).

### 5.2.3 Transport

#### Dumping

Numerous papers reported fecal sludge being discharged indiscriminately into streets, sewers, drains, nearby surface water, and coastal areas (15,23,31,71,73,78–80). The specific location of sludge disposal of was not reported in all studies.

Multiple factors influence the ultimate fate of emptied sludge, including the emptying method used by the households. Mechanical emptiers are equipped to transport fecal sludge longer distances to nearby disposal sites or treatment facilities (if they are available). However, manual emptiers must transport sludge in hand carts or buckets, which often leads to sludge emptied being buried on-plot, washed away in surface run-off, or discharged into nearby streams and rivers (15,23).

In Nairobi, there is no designated disposal site for fecal sludge; however it is legal for mechanical emptiers to discharge fecal sludge into the sewer network (23). Sludge from pits emptied through “flooding” (also referred to as “gravitational emptying”) is normally disposed into nearby rivers and drains, while sludge from manual emptying is either buried, discharged into surface water or drains (23). Surinkul and Koottatep (81) reported that private emptiers in Thailand disposed of sludge on abandoned land. In Vientiane, only seven of the 17 service providers exclusively disposed of sludge at official disposal sites, while six disposed of sludge at the disposal sites and open fields (47). Table 8 summarizes the reported percentages and/or volumes of untreated fecal sludge and the place of discharge from reviewed studies.

**Table 8. Reported fraction and/or volume, and location of fecal sludge discharged untreated into the environment.**

Country	City/Region	% disposed untreated	Volume discharged untreated (m <sup>3</sup> /day)	Location of disposal	Reference
Senegal	Dioukhop, Dakar suburb	74%		Streets	(73) as cited in (74)
Bangladesh	Dhaka	22% (of cases studied)		Drains or surface water	(79) as cited in (78)
Ghana	Accra		750 (39,000 m <sup>3</sup> /yr in 2000)	Ocean	(76) (82)
Indonesia	Jakarta	26% (surveyed households)		Surface water of gutters	(83)
Uganda	Tanzania	18%	130	Environment	(84)

Three studies reported high compliance of legal discharge of fecal sludge by vacuum tankers in Kumasi, Ghana (11,77,85). The studies credited the District Assembly who has strictly enforced regulations regarding the dumping of fecal sludge at non-designated sites (11).

Some studies focused only on a few steps in the sanitation chain, leaving uncertainty as to the ultimate fate of fecal sludge. Appiah-Effah et al. (64) studied the containment and emptying behavior of three districts in Ghana. Although only 1.9% of households reported emptying their pits, all were desludged mechanically with the contents reportedly transported them to a disposal site outside of the community. The study indicated the sludge was treated, but did not specify the process or where it was eventually disposed (64). Boot et al. (21) reported bucket latrine emptiers carried buckets of emptied fecal sludge to transfer stations without elaborating on the following chain in the sanitation process. A study of 15,983 households in South Africa reported 83.8% manually emptied their urine-diverting dry toilet, and although the intervention recommended households bury the sludge, the study did not explicitly confirm the behavior (75).

Disposal of fecal sludge onto land is not always indiscriminate; it is often applied directly to land for use as fertilizer. Jeuland et al. (77) qualitatively reported in Mali, some farmers pay for vacuum tankers to discharge fecal sludge onto their fields as fertilizer. Similar observations were made in Dakar (15), where sludge was purchased and used for gardening, and in Viantene, Lao PDR (47), where sludge was occasionally sold to farmers and pond owners. An observational report stated that 90% of collected fecal sludge in the Tamale Municipality was used as fertilizer (86). Another study examined the agricultural practices of 90 farmers in northern Ghana who used fecal sludge as fertilizer (87). While the study did not report statistics on treatment methods employed by the respondents, the authors stated that farmers either allow a 3-4 month drying time or several months of composting before applying the sludge to fields (87).

In Asia, fecal sludge reuse for agriculture and aquaculture is widely practiced. In cities within China, an estimated 30 million tons of sludge are collected and reused, mostly without any form of treatment (7). A study of 75 households engaged in farming activities in Vietnam found 70 households used latrines as their sanitation facility (88). Of those households, 85% utilized fecal sludge from latrines as fertilizer with almost all (98%) households reporting that they composted it before application (88). The length of reported composting time varied between households, with 9 composting excreta for less than one month, 23 composting for 1-3 months, 17 composting for 3-6 months, and 11 composting for six or more months (88). Another study in Nepal reported that fecal sludge from urine diverting dry toilets (UDDTs) was used for fertilizer without further treatment, however the length of time between emptying and reuse was not reported (80).

### Disposal sites

In areas where treatment facilities are not available, collected fecal sludge can be legally disposed of at designated “disposal sites” (8,47,87,89). Use of disposal sites displaces fecal sludge from households to the exterior fringes of cities and often results in 100% of collected waste being returned to the environment (8,89,90). This practice may be considered safe or unsafe, depending on the likelihood of human exposure at the disposal site or the probability of groundwater or surface water contamination. At the Korle Gono disposal site outside of Accra, vacuum tankers may legally discharge fecal sludge to the land and directly into the ocean (21).

Distance to disposal sites or treatment facilities can also be a major deterrent for vacuum tanker operators (9,47,77). Congested urban areas with heavy traffic and short operating hours of treatment facilities can lead vacuum tanker operators to discharge fecal sludge illegally (7,74). Data from Collignon et al. (91) and Jeuland et al. (77) revealed the varying extent of collected fecal sludge that is ultimately transported to a disposal or treatment facility (Table 9). One report indicated that the practice of illegal dumping by vacuum tankers only occurs when disposal sites and/or treatment facilities are closed, which typically includes weekends (15). Thus vacuum tankers have been known to discharge fecal sludge into nearby sewer drains in order to empty more houses within a day (7,23).

**Table 9. Percentage of collected fecal sludge discharged at disposal sites.** Adapted from (91).

Country	City	FS collected (trips/year)	% of trips ending at dumping site	Destination	Reference
Tanzania	Dar es Salam	100,000	7%	WWTP <sup>1</sup>	(91)
Senegal	Dakar	67,525	74% 65%	Fecal sludge collection station FSTP <sup>2</sup>	(91) (92)
Benin	Cotonou	26,667	75%	FSTP	(91)
Uganda	Kampala	7,000	42%	WWTP	(91)
Ghana	Kumasi	NA	95%	Treatment plant	(77)
Ghana	Tamale	NA	83%	Disposal sites	(87)

<sup>1</sup>WWTP – Wastewater treatment plant

<sup>2</sup>FSTP- Fecal sludge treatment plant

### **Transfer stations**

In order to alleviate the problem of hauling distances, a few cities have developed a system of “transfer stations” throughout the city where emptiers can dispose of fecal sludge (21,93). These transfer stations serve to reduce the distance between operators and treatment facilities. Vacuum tankers then collect the fecal sludge from the transfer stations and deliver it to a treatment facility; however, one study reported fecal sludge is not consistently collected and can overflow at transfer stations (21).

In areas where fecal sludge collection and treatment services are not available, communities have begun piloting different social enterprise structures. Wall et al. (94) reported on a successful pilot that used social franchising to launch pit latrine emptying businesses modeled after an earlier pilot in rural schools. The pilot was located in the Govan Mbeki community within the Amatole District Municipality in South Africa. The franchises developed multiple methods to empty the pits based on the accessibility of the pit. The teams emptied the latrines by hand and transported the fecal sludge in 220-litre, sealable drums. A disposal site next to a wastewater treatment plant (WWTP) was identified since it was located less than a kilometer away from the community. Similar to the urban school pilot, the disposal site was roped off and solid fecal sludge along with solid inorganic waste was discharged following the “latest guidelines and research about the depth of pits and how waste is handled and disposed of,” however the study did not report average disposal volumes of fecal sludge or the size of the disposal site.

### **5.3 Septic systems**

Septic systems treat wastewater close to the source and do not require the infrastructure of centralized wastewater transport and treatment. For this reason, septic systems are common in rural areas of many developed countries as well as urban and rural areas in developing countries (95) (Table 10). Septic systems are estimated to serve 80% of urban populations in Asia-Pacific countries (96).

**Table 10. Septic system coverage in countries and major cities.**

Country/city	Septic system coverage (%)	Reference
Australia	12	(97)
Egypt	49 (rural) 7 (urban)	(98)
Greece	14	(99)
Ireland	33	(100)
New Zealand	20	(101)
Nigeria	46	(102)
Turkey	28 <sup>a</sup>	(95)
United States	25	(6)
Accra, Ghana	40	(103)
Bangkok, Thailand	25	(104)
Colombo, Sri Lanka	33	(104)
Dakar, Senegal	58	(15)
Dar es Salaam, Tanzania	15	(105)
Hanoi, Vietnam	63	(106)
Ho Chi Minh City, Vietnam	79	(104)
Jakarta, Indonesia	39	(104)
Karachi, Pakistan	50	(104)
Kathmandu, Nepal	70	(104)
Kuala Lumpur, Malaysia	20	(107)
New Delhi, India	40	(104)
Phnom Penh, Cambodia	37	(104)
Yamoussoukro, Côte d'Ivoire	90	(108)

<sup>a</sup>Percent of municipalities using septic systems

### 5.3.1 Containment

Conventional septic systems consist of a septic tank that contains wastewater and a drain field that disperses wastewater effluent from the tank into the soil (109). Newer septic system technologies exist with innovative wastewater treatment designs, but since these systems are generally more expensive and less common, they are considered outside the scope of this review.

#### System failure

Hydraulic failure is observable to owners and occurs when the drain field becomes overloaded or clogged (109,110). Some of the most common signs of septic system failure include surface discharge and odors (111). Septic systems are generally expected to have a lifespan of 12-20 years (112), but over half of the septic systems in the US are over 30 years old and are reportedly more likely to fail (6). Estimated failure rates from US states are listed in Appendix A, Table 1. A study of septic systems in three communities in North Carolina found 18% of households surveyed had failing septic systems (113). The majority of systems in the area were over 20 years old (62%), and 15% of systems had unknown age, but the relationship between system age and failure was not analyzed in the study (113).

Failing septic systems provide little to no wastewater treatment. It is estimated that 10-20% of all onsite systems in the US are not providing adequate wastewater treatment (6). A study in Carteret County, NC found that monitored wells near a failing septic system exhibited microbial

contamination; average total coliform concentrations ranged from  $2 \times 10^3$  –  $1 \times 10^4$  MPN/100 mL, and enterococci were detected at least one time in each well with an average concentration of 100 MPN/100 mL (114). Following the system repair, total coliform levels were significantly lower, although average enterococci concentrations remained elevated (50 MPN/100 mL) (114). A separate study in the same area found that enterococci and *E. coli* concentrations were higher in wells near failing systems than near functioning systems (115). Among the two functioning systems, Rhodamine WT and MS2 virus tracers were infrequently found in low concentrations in wells near the effluent distribution box; for the two failing systems, tracers were found in higher concentrations and were detected in wells further from the distribution box (115).

Most septic system failures are caused by lack of maintenance or poor siting (111). Required maintenance may include desludging or needed repairs, whereas poor siting may be due to inappropriate soil type, inadequate sizing, or close proximity to groundwater (111). One study reported that failure rates for septic systems near water bodies was much higher than failure rates upland (116), which may be attributed to saturated subsoil.

#### System failure and sites

Five studies examined suitability of residential properties for septic systems. One study used a GIS-based soil rating system to assess soil suitability for septic systems in Alabama, where 44% of households use septic systems that malfunction at a rate of 20% (117). Using criteria based on the Alabama Onsite Sewage Disposal Rules that included percolation rate, depth to restrictive layer (e.g. groundwater table or dense soil), depth to seasonal groundwater table, slope, and flooding, over half the study area was unsuitable for conventional on-site wastewater treatment (118). Thirty-one percent of the study area was marginally suitable, and only 15% was found suitable (118). Additionally, many of the septic tanks in the area were assumed to be 20-30 years old, based on house ages (118), making them potentially more likely to fail.

A study in Ohio classified soil types in plots with septic systems. “Severe” soils displayed one of the following characteristics: they were wet, had slow permeability, had shallow bedrock, an inappropriate slope, or were susceptible to flooding or ponding (119). The study authors found that 63% of the septic systems surveyed were installed in severe soils, which had a significantly higher failure rate (23.3%) than the failure rate in all systems (16.2%) (119). Septic systems in soils that were wet, less permeable, and had slow permeability had significantly higher likelihood of failure than systems in soils with inappropriate slope or susceptibility to flooding or ponding (119). Systems in soils that were rated as “slightly inadequate,” in contrast, had a significantly lower failure rate (2.8%) (119). This study also found that systems installed in areas with high seasonal water tables were slightly more likely to fail (119).

Soil type was classified in a study in Virginia to assess site suitability for septic systems. The authors found that 41% and 42% of surveyed drain fields were in marginal and unsuitable soils, respectively, while only 17% of the drain fields were located in suitable soils (120). Drain fields in unsuitable soils repeatedly failed throughout the 16-month study period (120).

The remaining two studies found higher rates of site suitability for septic systems. A study in Mississippi sampled six septic system sites and found that all of them complied with state standards

for installation of drip irrigation, sprinkler irrigation, or mounds, depending on soil type (121). An analysis of septic system repair permits in Pennsylvania found that over half of the permits from a given year were from mechanical failures and less than 20% of permits were issued for improper site selection or installation (122). The researchers surveyed homeowners receiving permits and found that over 60% of repairs were reportedly on systems installed prior to 1972, when Pennsylvania enacted septic system regulations (122).

#### System failure and maintenance

Data on system maintenance was collected and reported in four studies. A study in Eudlo Township, Australia found of 48 septic systems surveyed, seven were found to be well-maintained and 32 tanks needed to be emptied at the time of the survey (97). In Lough Melvin, Ireland, 23 of 50 systems surveyed were over 20 years old and had undergone limited maintenance (123). Most of these systems did not have an absorption field and drained to a cesspit (123). A survey of 236 systems owners in Flanders, Belgium found that nearly 93% of respondents did not perform maintenance in the first three years of system operation, and that by five years, half of the system pumps had been replaced, but no other maintenance had been performed (124).

Maintenance contracts for septic system services were offered in Flanders, although they were costly (124). Five of 23 on-site wastewater systems surveyed had maintenance contracts, six were serviced with minimal maintenance (e.g. desludging or replacing a mechanical part), and 12 received no maintenance (125). Systems with minimal or no maintenance had similar treatment performance, but the five systems with maintenance contracts had nearly half the average effluent levels of chemical oxygen demand (COD), biological oxygen demand (BOD), and suspended solids (SS) of the 18 systems without contracts (125).

One study reported observations of septic system structural issues. In a survey of 48 systems in Australia, Ahmed et al. reported that 72% of systems had soggy absorption fields, 8% had structural problems such as broken lids, and 6% had insufficient capacity for the household (97).

#### Non-conventional septic systems

Conventional septic systems have tanks with two or three chambers. Several studies reported use of septic systems with single chambers or alternative designs. Septic tanks are prevalent in Hanoi (106), but reportedly the only conventional septic tanks in the city are those built by the French colonialists in the mid-nineteenth century (126). Some households' septic tanks may consist of a retaining chamber, a pit, or even a repurposed bomb shelter (126). In Dagupan City, Philippines, a survey of 1,200 residents found that 43% of households have single vault septic tanks that provide limited treatment and do not meet city standards (127). Tanks with multiple chambers have greater solids retention and can provide better microbial treatment (58).

Reviewed studies revealed that septic tank designs and functionality vary across regions. Non-conventional septic tanks commonly used in developing countries are listed in Appendix A, Table 2. Septic tanks are prevalently used in the Kathmandu Valley of Nepal and serve about 21% of the population, although most of the tanks are not truly "septic tanks;" they are larger pits lined with a brick wall and covered with a concrete slab (13). Most of them are poorly constructed and thus do

not operate as a septic tank (13). About half the tanks examined had two or more chambers, while 47% had single chambers and 4% were concrete ring tanks (13).

Similar nonconventional tanks were reportedly used by households in several studies. Baetings et al. found that among 520 households with septic systems, half of the households had a pit directly under the toilet, 22% had septic tanks, and 18% had offset pits (14% with one pit and 4% two pits) (47). In Muaeng Klong Luang, Thailand, 27% of households surveyed used conventional septic systems; the remaining households used one (41%) or two (32%) bottomless concrete ring tanks, which allow seepage of tank contents into the surrounding soil (128). Harder et al. reported that 15% of the septic tanks surveyed in Dagupan City, Philippines were bottomless or lacked concrete flooring (127). Bottomless, two-chambered septic tanks are also commonly used in Tuvalu (129).

### **Groundwater contamination**

Hydraulic failure can be readily observable to households, but treatment failure is more difficult to detect. Removal of pathogens and organic compounds is decreased when the drain field and subsoils become anaerobic and when effluent moves freely through cracks in the soils, escaping treatment. Treatment failure can lead to groundwater contamination, particularly in areas with a high groundwater table. Consequently, areas with high septic system density are more likely to have poor groundwater quality (130). The combined effluent may raise pathogen and nutrient levels in groundwater, and with more systems in a given area, there is a greater probability that one or more systems are either failing and discharging untreated effluent or are too close to the groundwater table (131).

Seven studies sampled groundwater near septic systems in developed countries. Five studies reported groundwater contamination in areas with dense or nearby septic systems, and two studies did not find clear evidence of groundwater contamination by septic systems. A study in residential Mississippi found no significant differences in contaminant concentrations between wells upstream and downstream of septic systems (121). Low levels of fecal contamination were found in well samples in South Australia, and septic systems were not a confirmed source of contamination (132). This study also did not find a statistical relationship between fecal coliform concentration and distance between septic systems and wells (132).

Two studies reported high microbial concentrations in areas with dense septic systems and high groundwater tables. In an septic-system-dense area of Queensland State, Australia where the water table was generally less than 1 m from the surface, groundwater samples had average fecal coliform concentrations of 25 cfu/100 mL and higher (133). Stewart et al. also found that fecal coliform concentrations in shallow wells within 30 m of septic system drain fields were higher when the water table was as high as or higher than the drain field level ( $\sim 10 - 5 \times 10^4$  MPN/100 mL) than when the water table was low ( $\sim 0 - 5 \times 10^2$  MPN/100 mL) (134).

Five studies reported contamination of wells in areas with dense septic systems. Two of these studies were conducted in a semi-rural community near Christchurch, New Zealand with domestic wells and septic systems on each plot (135,136). Fecal coliform concentrations were infrequently detected in low concentrations in the first study, but over half of the samples had total coliform concentrations above 10 CFU/100 mL (135). In the second study, indicator microorganisms were

found in a third of the 120 wells sampled (136). Coliforms were detected in 30-60% of shallow wells in Frederick County, MD, and the highest concentrations were found on small plots under a half acre with septic systems (137). Four of 50 wells across areas of Wisconsin with dense septic systems were found positive for Hepatitis A, rotavirus, poliovirus, or NLV; however, virus occurrence was only found in one of four samples for three of the wells (138). Results from a study of 60 wells near septic systems in Florida showed a relationship between septic system distance to well and *E. coli* counts in both the wet and dry seasons (139).

Rainfall effects were reported in six studies investigating septic systems as potential groundwater contamination sources (114,115,133,139–141). During the wet season in Florida, approximately 70% of the wells sampled had fecal coliform levels in the medium to high-contamination range (>500 CFU/100 mL), whereas only 40% of wells exhibited this level of contamination in the dry season (139).

Two studies reported on groundwater and surface water contamination by septic systems in developing countries. A study in Dar es Salaam sampled 25 dug wells in a neighborhood with septic systems on each plot and found fecal coliform levels between  $10^5$ - $10^6$  CFU/mL (142). The drain fields in the study neighborhood were found to extend below the water table level (142). In Mar del Plata, Argentina, 40 domestic and 10 deep wells were sampled in a residential area with a septic tank or cesspool on each lot, often within 3-10 m of wells (143). Fecal coliforms were found in 60% of the samples (143).

Findings from literature indicate that viruses and bacteria can travel great distances through unsaturated and saturated soils. Bacteria from septic tank effluent reportedly traveled beyond 28 m in one study (144). Hepatitis A and *Salmonella* were detected in wells 3 m and 64 m from septic tanks, respectively (145,146). A study investigating septic tank contamination of marine waters in Florida recorded virus migration rates between 0.57-24.2 m/h in subsurface limestone (147). Study authors suggested that the high migration rates could reflect rapid virus transport through fissures in limestone or rapid subsurface flow following recent a rainfall event (147). Two literature reviews were found that contained data on bacterial travel distances in soil following contamination from pit latrines, septic systems, infiltration beds, or subsoil injection (52,148); findings from these reviews are listed in Appendix A, Table 3.

Effluent from septic tanks can also affect surface water quality. Surface water contamination has been reported in areas with high septic system density (149), and like groundwater, contamination is greater when the water table is high (150). A study in Tuvalu found that *E. coli* from bottomless septic tanks travelled through groundwater during ebbing tides and contaminated coastal waters (129).

Areas with septic systems exceeding minimum setback distances or in unsuitable soils may be more vulnerable to surface water contamination (149). However, this was not observed in included studies. A study in Virginia found that groundwater within 10-20 m of septic system drain fields had fecal coliform bacteria densities near or below minimum detectable levels (MDLs) (151). In a study of 120 households with septic systems in North Carolina, 18% of wells in the study area were under 50 feet from septic tanks or drainage lines, well under the minimum county setback distance of 100 feet

(113). Little surface and groundwater contamination was found in the study area, although water from private wells had a higher number of samples positive for fecal indicators than public drinking water (113).

### 5.3.2 Emptying

#### Accumulation

Estimating household wastewater generation can provide useful guidelines on septic tank emptying frequency. Wastewater generation per capita depends on a number of factors, including lifestyle, time of day, climate, and year (152). Accumulation in septic tanks has been shown to increase gradually for two years, stabilize, and decrease after the third year due to anaerobic digestion (153). Mean sludge accumulation rates reported in literature are presented in Figure 1.

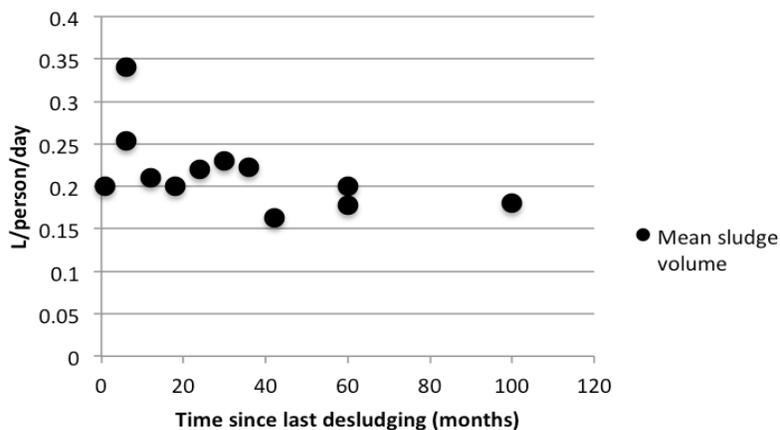


Figure 1. Mean sludge accumulation rates in septic tanks over time in literature. Source: (153–155).

#### Desludging behavior

While the liquid portion of septic tanks is diverted to a drain field or, in some cases, a drain or sewer, the tank sludge must be regularly emptied to ensure system functionality. This is commonly referred to as “desludging.” Household sludge accumulation will vary by number of users, septic tank size, volume of wastewater generated, and volume of solids, but recommended desludging frequency in the United States is every 3-5 years (156).

Septic tank desludging in practice, however, is often infrequent, and there is limited literature on septic tank desludging behavior. Eight studies reporting septic tank desludging behavior, knowledge, and beliefs were found. Of these studies, one was conducted in Canada, two in the US, one in Ireland, one in Laos, two in Vietnam, and one in the Philippines.

Of the eight studies on septic tank desludging behavior, three studies reported results on mean, median, or range of septic tank desludging among surveyed households. An evaluation of 75 septic tanks in Eastern Ontario found that the median reported years since the tank was emptied was five and four years for tanks with and without water softener, respectively, but reports ranged from six months to 20 years (157). Similarly, a study in Hanoi, Vietnam found that the reported years since

household septic tanks were emptied ranged from 1-20 across 20 households (32). Another, larger study in Hanoi with 692 households found that the median and mean desludging period was reportedly seven and eight years, respectively (158). While many households routinely empty their septic tanks every several years, other households wait decades pass between desludging; however, these desludging periods are self-reported.

Seven study authors reported desludging frequency using telling cutpoints (Table 11). In Vientiane, approximately half of the tanks in the study had never been emptied (47). A survey of on-site systems in Hawaii found that 80% of households either had never desludged their systems or did not know the last time they were desludged (159). Study authors surmised that the system owners likely had never desludged their systems (159). Over half of the respondents in a survey of 50 systems in Ireland could not recall when the last time they have desludged their tanks (123).

**Table 11. Time since last septic tank desludging, as reported by system owners.**

Study area	N	Frequency emptied	Percent respondents (%)	Reference
Mount Gambier, Australia	29	Past 4 years	76	(132)
Lough Melvin catchment, Ireland	50	Past 5 years	30	(123)
Dagupan City, Philippines	850	Past 10 years	13	(127)
Palm Bay, Florida	60	Never	75	(139)
Hawaii, US	288	Never	80	(159)
Hanoi, Vietnam	692	Never	89.6	(158)
Ba Ra, Vietnam	100	Never	39	(8)
Son La, Lang Son, Hoa Binh, and Bac Ninh, Vietnam	400	Never	80-89	(8)

### Limiting factors for desludging

Kaminsky et al. conducted a literature review to identify important factors contributing to septic systems sustainability and then held an expert elicitation to identify the most important factors (160). Findings from the expert consensus are listed in Table 12. Several of these themes surfaced in the literature included in this review. Physical access to the tank also was mentioned in one study; Bassan et al. reported that many septic tanks in Vietnam are below houses, so the floor must be broken to access and desludge the tank (8).

**Table 12. Important factors impacting on-site wastewater treatment sustainability.** Source: (160).

<b>Factors negatively impacting sustainability</b>	<ul style="list-style-type: none"> <li>• Poor installation quality;</li> <li>• Cost of desludging;</li> <li>• Difficult or inconvenient to dispose of sludge; and</li> <li>• Poor quality of materials</li> </ul>
<b>Factors positively impacting sustainability</b>	<ul style="list-style-type: none"> <li>• Presence of local, for-profit businesses for services such as installation, desludging, repair, and supplying spare parts;</li> <li>• Follow-up programs after system construction;</li> <li>• Presence of local NGOs or nonprofits in sanitation; and</li> <li>• Owner occupancy in household with system</li> </ul>

### Septic system desludging knowledge and beliefs

Eight studies reported household beliefs, practices, or knowledge regarding septic tank desludging. In three studies, many respondents reported that septic tanks were only emptied when they were clogged, overflowing, or damaged (8,13,127,158). Only 6% of respondents in a baseline study in Thimphu City, Bhutan thought septic tanks should be emptied regularly, and 60% did not know how often they should be emptied ((161) as cited in (162)). Fink found less than half of the respondents in Hanoi knew how to empty their septic tanks or knew if their tanks needed to be emptied (126).

A qualitative study by Halcrow et al. in Thimphu City, Bhutan, used in-depth interviews with users and non-users of septic tank desludging services to investigate their knowledge, beliefs, and attitudes surrounding desludging (162). The authors found most users and non-users reported looking in the inspection chamber for liquid depth, but did not know how to determine if their septic tanks were full and did not know they needed to examine sludge levels. Users and non-users differed in that non-users were less aware of available desludging services, but across both groups, most users felt they did not need to desludge their tanks unless they were overflowing (162).

In Vientiane, the majority of respondents knew that their storage may be full if the toilet became blocked, and only 5% of the respondents who had never emptied their tanks did not know how they could tell if their septage storage was full. But only 6 of 520 households regularly checked their septage storage or tanks to determine if they were full (47).

Lack of information or education can lead to improper system maintenance. Moelants et al. found 35% of system owners surveyed did not receive maintenance information from their system manufacturers. For those who did receive information, the type of waste allowed in the system was most frequently explained, however information on desludging and other maintenance were seldom provided (124).

### Septic system desludging and ownership

The effect of owner occupancy on desludging raised by Kaminsky et al. (160) did not surface in the literature, but three studies reported that households shared septic tanks (47,126,163).

### Septic system desludging costs

Cost of desludging services was discussed in six studies. Halcrow et al. found that most system owners in Thimphu City were willing to pay the city fee for desludging services (162). However, residents outside of the city center who had to pay for additional desludging transportation costs found the desludging and transportation fees expensive (162). Reportedly, desludging services in Danang, Vietnam are not affordable for many households, even though six service providers operate in a competitive market (164). In Flanders, Belgium, maintenance contracts were available for 196.82€ a year, and only 37% of 236 system owners surveyed in the area had these contracts (124).

Desludging costs in Kathmandu Valley were provided in a report by the High Powered Committee for Integrated Development of the Bagmati Civilization (13). Desludging by private and public providers costs households between 1500-2500 NRs (15-25 USD), depending on distance travelled, and manual desludging fees begin at NRs. 2000 (20 USD) (13). The authors did not discuss whether these fees

are widely affordable to different user groups, but did report that fees for private providers and manual emptying providers are negotiated based on travelling distance as well as client economic status (13).

Average vacuum tanker emptying fees in Vientiane were estimated at LAK 210,000 (26 USD), but the study authors did not report whether these fees are affordable in the region (47). Volumetric rates are used in Malaysia, where private operators charge \$77 to desludge tanks under 2 m<sup>3</sup> and the Indah Water Konsortium (IWK) charges \$100 (165). Both providers charge \$38 for each additional cubic meter and \$18/m<sup>3</sup> for sludge treatment and disposal (165). In Muaeng Klong Luang, Thailand, the costs of fecal sludge collection and disposal over the facility lifespan for one-pit, two-pit, and commercial septic systems were estimated as 240, 300, and 200 Baht, respectively, (128). The commercial septic system had greater storage capacity than the pits and therefore had lower emptying costs, but study authors did not comment on affordability (128).

### 5.3.3 Transport and disposal

Another factor affecting household disposal of septic tank sludge is availability of desludging services. In cities lacking sufficient or affordable desludging services, septic tank sludge is disposed directly into the environment. Twelve articles reported on desludging service availability and household behavior.

Dagupan City reportedly has no sludge treatment or disposal facilities, so of the 850 households surveyed by Harder et al., 30% manually desludged and buried waste in a pit, 13% emptied waste into water bodies, 5% emptied waste onto farmland, and 7% of households emptied sludge anywhere (127). In Dhaka, Bangladesh, households served by septic systems often empty their tanks manually or dispose of wastewater in low-lying lands, natural drains, water bodies, or through surface drains (104). In Mira-Bhayandar and Wardha, India, only one vacuum suction tanker services each city, so only a small fraction of septic tanks in the cities can be emptied each year (163).

Even when desludging services are available, system users may not utilize them. Six private companies provide desludging services in Danang, Vietnam, where the majority of households use septic tanks, yet their fees are high and households dispose of their effluent in the street and surface drains (164). A cross-sectional study of 41 households in Hanoi found that less than 20% of respondents used the public service, URENCO, or a private service provider to desludge their tank (126), although the authors postulated that URENCO did not have the resources to meet consumer demand. Instead of desludging the tank, over 20% of households surveyed used Bio-powder, a low-cost mixture of organic compounds that digests the tank sludge (126).

Safe desludging was reportedly more prevalent in three studies. Ten private companies provide desludging in Kathmandu Valley, and their services are used by 18% of septic tank owners. Sixteen percent of respondents use public services, 34% hire local service providers to manually empty the tanks, and 28% emptied their own tank (13). Households in farming communities often emptied their own tanks to use the sludge as manure (13). In Muaeng Klong Long, Thailand, 20% and 33% of respondents used desludging services from private and municipal providers, respectively; 47% did not use desludging services, but the study authors did not specify if the tanks were emptied using other

methods (128). Septic tank desludging in Yamoussoukro, Côte d'Ivoire is typically done by private service providers, but is sometimes done by manual emptying (108).

Seventeen private operators service Vientiane, Lao for desludging septic systems (47). Only 3 of the emptied septage tanks or pits in the study had been desludged manually; 99% were emptied by vacuum tankers (47). Desludging was reportedly more frequent in the rainy season than the dry season (47).

Private companies provide desludging services to Son La, Ba Ria, Hoa Binh, Bac Ninh, and Lang Son, Vietnam but discharge the waste directly into the environment because there are no designated discharge sites (8). In Paramaribo, municipal sewage tankers desludge tanks and discharge the sludge into the Surinam River, a government-designated disposal site, or directly into the environment when the road conditions are poor (166). In Ireland, farmers commonly desludge septic tanks and reuse the sludge as fertilizer (167).

Conventional septic systems feature an absorption field, but a common practice in some areas is to dispose of effluent via drains or canals. A rapid assessment in Vientiane, Laos found that none of the septic tanks surveyed had drain fields or soak pits (47). In Vientiane, the Vientiane Urban Development and Administration Authority recommends for septic effluent to be discharged into drains (47). In the following survey of 528 households, 17% of the septage systems discharged effluent directly into the environment: 13% discharged into open drains, 3% discharged into open water, and 1% discharged onto the ground (47). In Klong Luang, Thailand, most households reportedly have pour-flush latrines connected to septic tanks, which discharged wastewater into canals or open bodies of water (128,168). These practices can introduce contamination into water sources and recreational bodies of water. In cities like Manila and Maynilad, septic tank effluent is discharged into sewers and treated at the wastewater facilities (169).

### 5.3.4 Treatment

#### *Contamination reduction in the septic tank*

When regularly maintained and desludged, septic tanks provide effective on-site wastewater treatment. The extent of pathogen removal from sludge in the anaerobic tank is largely dependent on retention time and emptying frequency, and typically can range from 0-2 log reduction for viruses, bacteria, protozoa, and helminths (29). Six studies included in the review reported data on pathogen removal rates from effluent in the septic tank (Table 13). Fifteen studies reported mean pathogen concentrations in sludge or effluent; these findings are reported in Tables 14 and 15. Tsuzuki et al. estimated per capita discharge of fecal and total coliforms for on-site wastewater treatment plants in Bangkok (170); study results are listed in Appendix A, Table 4.

**Table 13. Septic tank treatment reported in literature.**

Pathogen	Unit	Tanks sampled (N)	Sludge concentrations (mean and range)	Effluent concentrations (mean and range)	Removal rate (%)	Reference
Fecal coliforms	10 <sup>6</sup> /100 <sup>6</sup> m	3	15.0 (13-17)	5.2 (4.8-5.7)	65.3 <sup>a</sup>	(102)
Fecal coliforms	10 <sup>6</sup> /100 <sup>6</sup> m	3	20.3 (17-24)	10.7 (7.4-14)	47.2 <sup>b</sup>	(102)
Fecal coliforms	10 <sup>6</sup> /100 <sup>6</sup> m	3	6.7 (5.7-7.8)	3.6 (2.9-4.3)	46.3 <sup>c</sup>	(102)
Fecal coliforms	Count/mL	1	2.1 x 10 <sup>5</sup> (7.1 x 10 <sup>4</sup> - 6.3 x 10 <sup>5</sup> ) <sup>d</sup>	5.1 x 10 <sup>4</sup> (2.7 x 10 <sup>4</sup> - 9.8 x 10 <sup>4</sup> ) <sup>d</sup>	75.7	(136)
Fecal coliforms	Count/mL	9	2.6 x 10 <sup>4</sup>	1.4 x 10 <sup>2</sup>	99.5	(171)
Fecal coliforms	Count/mL	2	4.26 x 10 <sup>7</sup>	2.53 x 10 <sup>7</sup>	40.17	(58)
Total coliforms	Count/mL	9	5.2 x 10 <sup>6</sup>	1.1 x 10 <sup>5</sup>	97.9	(171)
Total coliforms	Count/mL	2	9.55 x 10 <sup>7</sup>	5.98 x 10 <sup>7</sup>	37.4	(58)
MS-2 coliphage	Pfu/mL	1	3 x 10 <sup>4</sup>	8 x 10 <sup>3</sup>	74.44	(172)
Parasite eggs	Count/L	9	1178	1.3	99.9	(171)
Hookworm	No. ova	4	4,500 (270-30,195)	639 (ND-4,500)	85.8	(173)
Ascaris	No. ova	4	9,765 (900-72,000)	1,080 (ND-6,300)	88.9	(173)

<sup>a</sup>After 24 hr detention time; <sup>b</sup>After 48 hr detention time; <sup>c</sup>After 72 hr detention time; <sup>d</sup>95% confidence interval

**Table 14. Reported pathogen concentrations in septic tank sludge.**

Pathogen	Unit	Tanks sampled (N)	Sludge concentrations (mean and range)	Reference
Somatic coliphage	pfu/g d.w.	20	1.3 x 10 <sup>6</sup> (ND-9.7 x 10 <sup>6</sup> )	(32)
Male-specific bacteriophage	pfu/g d.w.	20	2100 (ND-6200)	(32)
<i>E. Coli</i>	CFU/g d.w.	20	1.1 x 10 <sup>6</sup> (7200-6.2 x 10 <sup>6</sup> )	(32)
<i>Enterococcus</i> spp.	CFU/g d.w.	20	78000 (1500-4.0 x 10 <sup>5</sup> )	(32)
<i>Salmonella</i> spp.	MPN/g d.w.	20	570 (ND-1900)	(32)
Helminth ova	No. l <sup>-1</sup>	20	16000 (1000-50000)	(32)
Total coliforms	Count/100 mL	1	1.6 x 10 <sup>6</sup>	(155) <sup>a</sup>
Fecal coliforms	Count/100 mL	1	5.8 x 10 <sup>5</sup>	(155) <sup>a</sup>
Fecal coliforms	CFU/L <sup>-1</sup>	1	7.7 x 10 <sup>7</sup> (2.5 x 10 <sup>7</sup> - 1.2 x 10 <sup>8</sup> )	(174)
Fecal coliforms	Log <sub>10</sub> 100mL <sup>-1</sup>	28	6.7	(35)
Enterococci	Log <sub>10</sub> 100mL <sup>-1</sup>	28	6.5	(35)
Somatic coliphages	Log <sub>10</sub> 100mL <sup>-1</sup>	28	6.4	(35)

<sup>a</sup>Average measurement from second compartment of Orillia Hospital house tank

**Table 15. Reported pathogen concentrations in septic tank effluent.**

Pathogen	Unit	Tanks sampled (N)	Effluent concentrations (mean and range)	Reference
Total coliforms	CFU 100 mL <sup>-1</sup>	143	6.3 x 10 <sup>4</sup>	(175)
Thermotolerant coliforms	CFU 100 mL <sup>-1</sup>	29	1.3 x 10 <sup>4</sup> - >2.4 x 10 <sup>7</sup>	(132)
Fecal coliforms	10 <sup>4</sup> CFU L <sup>-1</sup>	7	17 (2-38)	(176)
Fecal coliforms	10 <sup>4</sup> CFU L <sup>-1</sup>	7	36 (10-312)	(176)
Fecal coliforms	10 <sup>4</sup> CFU L <sup>-1</sup>	7	52 (10-202)	(176)
Fecal coliforms	10 <sup>4</sup> CFU L <sup>-1</sup>	7	19 (5-72)	(176)
Fecal coliforms	MPN/100 mL	25	5.8 x 10 <sup>4</sup> (5 x 10 <sup>3</sup> - 1.7 x 10 <sup>5</sup> )	(128) <sup>a</sup>
Fecal coliforms	Count/100 mL	2	2.5 x 10 <sup>5</sup> ((0.03-9) x 10 <sup>5</sup> )	(177)
Fecal coliforms	Count/100 mL	1	1.01 x 10 <sup>6</sup>	(178) as cited in (177)
Fecal coliforms	Count/100 mL	1	1.08 x 10 <sup>6</sup>	(155) <sup>b</sup>
Fecal coliforms	Count/100 mL	2	2.3 x 10 <sup>6</sup>	(179)
Fecal coliforms	Count/100 mL	-	4 x 10 <sup>5</sup>	(180)
Total coliforms	Count/100 mL	2	4 x 10 <sup>5</sup> ((0.02-1.7) x 10 <sup>5</sup> )	(177)
Total coliforms	Count/100 mL	3	5.63 x 10 <sup>6</sup>	(178) as cited in (177)
Total coliforms	Count/100 mL	1	2.6 x 10 <sup>6</sup>	(155) <sup>a</sup>
Total coliforms	Count/100 mL	2	3.7 x 10 <sup>6</sup> - 1.2 x 10 <sup>7</sup>	(179)
Total coliforms	MPN/100 mL	16	1.78 x 10 <sup>7</sup> ± 2.7 x 10 <sup>6</sup>	(167)
E. coli	MPN/100 mL	16	2.22 x 10 <sup>6</sup> ± 2.4 x 10 <sup>5</sup>	(167)
E. coli	Count/100 mL	2	1.2 x 10 <sup>6</sup>	(179)
F-RNA phages	Count/100 mL	-	1.01 x 10 <sup>6</sup>	(181)

<sup>a</sup>Two-chambered tank; <sup>b</sup>Average measurement from second compartment of Orillia Hospital house tank

Treatment efficacy within the tank can vary depending on climate. A study in Georgia, USA reported *E. coli* growth, rather than reduction, in a septic tank in hot, humid summer conditions.

Temperatures during sampling ranged from 29-33°C, maximum relative humidity ranged from 90-99%, and as a result, concentrations in the septic tank effluent were 100 times higher than the influent wastewater (182).

Many helminth eggs eventually settle into the sludge portion of the tank, therefore helminth concentrations are assumed to be higher in sludge than in effluent (183). A study sampling sludge from 20 septic tanks in Vietnam found all samples positive for helminth eggs, *E. coli*, and *Enterococcus* spp (32). Yet helminths have low infective doses (184,185), so lower concentrations of eggs can still present public health risks. A study in India found that among 33 effluent samples tested for pathogen viability, 77% and 91% had viable hookworm and *Ascaris* ova, respectively (173).

#### **Effluent treatment in drain fields and unsaturated soil**

Drain fields remove pathogens from effluent through distributing small quantities of effluent into the subsoil. Pathogen concentrations eventually decline in the subsoils due to attenuation, die-off, and predation (186,187). Unsaturated subsoils have been shown to achieve high reduction of total coliform bacteria, fecal coliform bacteria, and viruses after 60 cm (188) with up to 99% removal (187,189). Table 16 presents virus removal efficiencies reported in literature, as cited from Van Cuyk et al. (2004) and Lewis (1982).

**Table 16. Virus removal efficiencies reported in field studies on wastewater soil treatment.** Adapted from (52,189).

Soil type	Pathogen	Applied concentration	Depth	Removal	Reference
Fine loamy soil	Total coliform	4 x 10 <sup>6</sup> count/100 mL	0.15 m	88%	(190)
Fine loamy soil	Fecal coliform	4 x 10 <sup>6</sup> count/100 mL	0.15 m	95%	(190)
Mature absorption system soil	MS-2 PRD-1	1 x 10 <sup>5</sup> plaque forming units (pfu)/mL	0.6 m	99.9%	(189)
Unsaturated fine sand	PRD-1	6.0 x 10 <sup>10</sup> or 1.6 x 10 <sup>11</sup> pfu/mL	0.6 m	1.43-log removal <sup>a</sup>	(191)
Unsaturated fine sand	PRD-1	6.0 x 10 <sup>10</sup> or 1.6 x 10 <sup>11</sup> pfu/mL	0.6 m	1.91-log removal <sup>b</sup>	(191)
Unsaturated fine sand	MS-2	3 x 10 <sup>4</sup> pfu and 7.8 x 10 <sup>3</sup> pfu/mL <sup>3 c</sup>	0.3 m	99.17% <sup>d</sup>	(172)
Unsaturated fine sand	MS-2	3 x 10 <sup>4</sup> pfu and 7.8 x 10 <sup>3</sup> pfu/mL <sup>3 c</sup>	0.6 m	98.45% <sup>d</sup>	(172)
Unsaturated fine sand	MS-2	3 x 10 <sup>4</sup> pfu and 7.8 x 10 <sup>3</sup> pfu/mL <sup>3 c</sup>	1.52 m	99.79% <sup>d</sup>	(172)
Recirculating gravel filter	ΦX174	1 x 10 <sup>0</sup> – 1 x 10 <sup>4</sup> pfu/mL	-	1-log removal	(192)
Clay loam soil	ΦX174	1 x 10 <sup>0</sup> – 1 x 10 <sup>4</sup> pfu/mL	0.6 m	100%	(192)
Sandy loam	Fecal coliform	7.7 x 10 <sup>7</sup> cfu/L <sup>-1</sup>	0.9 m	91.1%	(174)
Sand filter system	Fecal coliform	7.7 x 10 <sup>7</sup> cfu/L <sup>-1</sup>	0.9 m	99.8%	(174)
Sand + silt loam	Bacteria	1.7 x 10 <sup>5</sup> count/100 mL	0.6 + 0.3 m	100%	(193) <sup>e</sup>
Sand loam	Bacteria	2.5 x 10 <sup>5</sup> count/100 mL	0.6 m	99.8-100%	(194) <sup>f</sup>
Loamy sand	Bacteria	5.1 x 10 <sup>6</sup> count/100 mL	0.6 m	99.999%	(195) <sup>e</sup>
Loamy sand	Bacteria	6.9 x 10 <sup>4</sup> count/100 mL	0.3 m	100%	(196) <sup>e</sup>
Fine sandy loam	Bacteria	2 x 10 <sup>5</sup> – 5 x 10 <sup>6</sup> count/100 mL	1.2 m	100%	(197) <sup>g</sup>
Sandy loam	Bacteria	1.1 x 10 <sup>6</sup> count/100 mL	1.2 m	100%	(198) <sup>g</sup>
Sandy clay	Bacteria	1.1 x 10 <sup>6</sup> count/100 mL	1.2 m	100%	(198) <sup>g</sup>
Clay	Bacteria	1.1 x 10 <sup>6</sup> count/100 mL	1.2 m	100%	(198)
Sand	Bacteria	1.3 x 10 <sup>3</sup> count/100 mL	1.5 m	98.5%	(199) <sup>h</sup>
Fine sandy soils	Various	0.06-43.7 MPN/L	3 m	ND <sup>i</sup>	(200)
Fine loamy sand	Enterovirus	1 x 10 <sup>3</sup> – 7 x 10 <sup>3</sup> pfu/100 L	3-9 m	99.9%	(201)
Fractured rock	Bacteria	7.7 x 10 <sup>5</sup> count/100 mL	3.5 m	11.7%	(202) <sup>j</sup>
Fine loamy soil	Total coliform	11 x 10 <sup>6</sup> pfu/100 mL	6.1 m	99.9%	(190)
Fine loamy soil	Fecal coliform	1.3 x 10 <sup>6</sup> pfu/100 mL	6.1 m	99.8%	(190)
Fissured chalk	Bacteria	2 x 10 <sup>6</sup> count/100 mL	15 m	93.5%	(203) <sup>h</sup>
Alluvial gravel	Bacteria	2.5 x 10 <sup>4</sup> count/100 mL	15 m	99.8%	(204) <sup>h</sup>
Fine loamy soil	Total coliform	70 x 10 <sup>3</sup> count/100 mL	12.5-28 m	93%	(190)
Fine loamy soil	Fecal	15 x 10 <sup>3</sup> count/100 mL	12.5-28	98%	(190)

<sup>a</sup>At low hydraulic loading rate; <sup>b</sup>At high hydraulic loading rate; <sup>c</sup>In raw wastewater and applied STE, respectively; <sup>d</sup>Percent removals based on concentrations in septic tank effluent applied to soil; <sup>e</sup>Laboratory column experiments; <sup>f</sup>Mound disposal system; <sup>g</sup>Lysimeter studies; <sup>h</sup>Land disposal of sewage; <sup>i</sup>Virus was detected at one site between 0.6-0.9 m; <sup>j</sup>Field studies

The efficacy of the drain field treatment is affected by the site's topography, soil drainage and aeration, climate, native vegetation, and biological activity in the soil (205). For example, treatment efficacy of two experimental drain fields in North Carolina was affected by their topography. The log<sub>10</sub> reductions for coliphages, fecal coliforms, and fecal streptococci were found to be 144%, 167%, and 190% higher (respectively) in the field with higher and drier conditions than the field with lower, wetter conditions (206).

Characteristics of the drain field subsoils are also major determinants of treatment efficacy. The depth, permeability, and composition of the drain field subsoils affect effluent dispersal and pathogen reduction (186). Fine-textured soil with high clay content provide better treatment than coarse-textured soils (207,208) due to better microbial adsorption (209). In contrast, areas with sandy soil or thin soil over creviced bedrock have been found to be susceptible to groundwater pollution by septic systems (52,210), although greater reduction of bacteria and viruses has been demonstrated in sandy soils with 15% or higher clay content (210).

## 5.4 Fecal Sludge Treatment

### 5.4.1 Capacity of fecal sludge treatment plants

Where FSTPs are available, they often do not have sufficient capacity to treat all the collected sludge, much less the total volume of sludge generated. In 1992, only 8.5% of registered households in Bangkok, were served by night soil treatment plants. At the time of the study, only two night soil treatment plants were in operation, with two additional plants to be constructed before 2000. However, by the authors' estimation, the total treatment capacity in 2000 would still only be able to treat 50% of volume generated in 1992 (211). In Dakar, Senegal and Addis Ababa, Ethiopia the estimated difference in treatment capacity and fecal sludge generation is 7,730 m<sup>3</sup>/day and 470 m<sup>3</sup>/day, respectively (15,165).

FSTPs may be overloaded due to the lack of adequate treatment capacity (78,92). In Dakar, Senegal, there are only two FSTPs in operation, both of which are operating over their designed capacity (15,92). While three FSTPs in Accra, Ghana were able to treat 90,000 m<sup>3</sup> of fecal sludge in 2000, the combined capacity was not sufficient to treat the remaining 70,000 m<sup>3</sup> of sludge (82). In some cases, such as in Palu, Indonesia, FSTPs operating under their designed capacity can lead to inefficient treatment (78). Overloaded FSTPs can lead to inefficient sludge treatment as well as frequent equipment problems (15).

In some cases, FSTPs and WWTPs may not be operated or maintained correctly, leading to inefficient treatment. A 1992 study of WWTPs in the Caribbean reported 22% of facilities using an activated sludge treatment process were operating incorrectly (212). They also found 23% of treatment facilities allowed sludge to accumulate in the treatment process, which led to reduced retention

times and thus reduced efficiencies (212). Lack of financial resources and proper operation and maintenance were cited as the causes for a failed FSTP in the Kathmandu Valley, Nepal (13).

#### 5.4.2 Sludge Treatment Efficiencies

Fecal sludge treatment processes can be divided into two categories based on whether or not they separate sludge into solids and liquids (213). While fecal sludge treatment literature often focuses on reducing biological oxygen demand, chemical oxygen demand, our discussion will examine treatment efficiencies with regards to pathogen reduction. During solid-liquid separation processes, helminth eggs typically settle in the solid fraction and are thus more concentrated in the sludge portion after separation (214). Other pathogens, such as bacteria and viruses, are distributed in both the liquid and solid streams (214). Table 17 summarizes the range in log reductions reported in the literature for the broad categories of bacteria, viruses, parasites, and helminths.

**Table 17. Pathogen log reductions for various sludge treatments.**

Treatment process	Bacteria	Viruses	Parasites	Helminths	Duration required for helminth reduction (in months)	Helminth references
Thickening tanks				<1	3 hours	(31)
Settling ponds				3 <sup>a</sup>	4	(215)
Planted dewatering drying beds (constructed wetlands)	1-2			1.5	12	(216)
				2-3	6	(217)
						(218)
Unplanted drying/dewatering beds (for pre-treatment)				6		(213)
				6	8-59 days	(219)
				1.2	11-30 days	(14)
Composting (windrow, thermophilic)	2-3	2-3	2-3	1.5-2.0	3	(220)
	2-4					(221)
	6					(222)
pH elevation >9				3	6	(223)
Anaerobic (mesophilic) digestion	1-2	1	0	0.5	0.5-1.0	(29)
	0.3-3	2.0-6.2	<0.3	0		(224) (225)
Aerobic digestion	1-2	1	0			(226) as cited in (227)
Air drying <sup>b</sup>	2-3	1-3	1-3			(226) as cited in (227)
Lime stabilization	2-3	3	0			(226) as cited in (227)

<sup>a</sup>Effects depend on moisture levels

#### Thickening tanks

Thickening tanks are used to separate fecal sludge into solid and liquid phases. In Accra, Ghana the Achimota FSTP used a twin thickening tank process followed by four stabilization ponds (31). After the thickening tanks, the amount of helminth eggs was reduced by 48% in the effluent (31). Fecal sludge effluent from the thickening tanks were then discharged into four stabilization ponds where fecal coliforms were reduced from  $10^6$  to  $10^4/100$  mL (31).

### **Unplanted drying beds**

Pilot studies of unplanted drying beds in Accra have fecal sludge dewatered to less than or equal to 40% total solids still containing helminth eggs in hazardous concentrations, however no recoverable helminth eggs were found at greater than or equal to 70% total solids (213). Storage time and temperature were important factors to ensure egg reduction. Similarly, the liquid effluent from the dry beds were reported to be free from helminth eggs. A more recent pilot study of unplanted drying beds in Kumasi, Ghana using a mixture of fecal sludge from public latrines and septage from septic tanks reported 100% removal rate of helminth eggs in the effluent from the drying beds (219).

Helminth eggs concentrations were still high in the dewatered fecal sludge (exact number not reported), therefore the sludge was subsequently co-composted along with biodegradable solid waste (219).

A more recent study of drying beds used in Dakar, Senegal reported a 93.9% reduction of *Ascaris* eggs (14). Fecal sludge was pre-treated using a settling-thickening tank and then applied to drying beds at 92-117 kg TS/m<sup>2</sup> yr and 150-175 kg TS/m<sup>2</sup> yr. After 11-30 days of drying, the final sludge product contained an average of 69 eggs/g TS (14), which is higher than the WHO guideline of <1 *Ascaris* egg/g TS for agricultural use of fecal sludges (38).

### **Planted drying beds**

Kengne et al. (217) studied the treatment efficiency of vertical flow constructed wetland drying beds loaded with fecal sludge from pit latrines, septic tanks, and public toilets. Fecal sludge was loaded over the course of six months at 100, 200 and 300 kg TS/m<sup>2</sup> yr. Helminth egg concentration of raw fecal sludge feed ranged between 4,167 to 22,267 eggs/L. After one month of storage, overall average helminth egg concentration was 79 eggs/g TS although viable egg concentration was 38.5 eggs/g TS. After six months of storage the average helminth egg concentration dropped to 7.5 eggs/g TS with only 4.03 eggs/g TS being viable.

Ghemandi et al. (218) conducted a literature review on studies examining the pathogen removal efficiency of surface-flow constructed wetlands. The review compiled data from 70 studies, mostly from North America (47), Europe (13), and Australia (6). Regardless of the type of pre-treatment, constructed wetlands showed a one to two log reduction in fecal indicator bacteria (total coliforms, fecal coliforms, fecal streptococci, and *E. coli*) for treated municipal sludge. The review concluded that constructed wetlands receiving previously disinfected wastewater may actually be a source of contamination rather than increase pathogen removal.

### **Anaerobic digestion**

Avery et al. (225) conducted a review of the pathogen reduction of anaerobic digestion processes. The review found substantial differences in the removal rates for indicator organisms (total coliforms and fecal coliforms) and pathogen occurrence. Table 5 in Appendix A contains characteristics from studies using sewage waste included in the review. Gram-negative enteric bacteria may be able to endure anaerobic digestion conditions better than other pathogens, thus it may not be a suitable indicator for pathogen reduction for digestion processes. For total coliforms, reduction ranged from 0.3 to 3 logs for sewage feedstock (225). Fecal coliform reductions ranged from 1.3-3 logs and *E. coli*

reductions ranged from 1 to 2.0 logs (225). Studies of other gram positive species such as *Clostridia* and *Campylobacter*, saw little reduction from anaerobic digestion (225).

A study with a feedstock of human night soil reported 3.6-6 log reduction of *Vibrio cholerae* (58). Enterovirus reductions were less than 2.0 logs, although 6.2 log removal of poliovirus was observed in one study (55). Protozoan removal was relatively ineffective with removal rates of no change to 0.3 log reduction observed in two studies (48, 61). The results of the reviewed studies showed an increase in die-off rates of mixed feedstock (sewage combined with cattle dung) compared to sewage only feedstocks. Interestingly, the results from the review indicate pathogen die-off was greater under psychrophilic conditions (<20 C) than mesophilic conditions (20-45 C), however the authors noticed the relationship of temperature on microbial reduction varied by pathogen (225).

### Composting and co-composting

Koné et al. (220) studied the pathogen removal efficiency for dewatering fecal sludge followed by co-composting. Fecal sludge obtained from public toilets and septage with 2-3% total solid content was loaded onto 25 m<sup>2</sup> drying beds and dewatered to >20% TS. The dewatered sludge was then co-composted with organic solid waste. Initial helminth concentration in the raw fecal sludge was 25-83 helminth eggs/g TS, which decreased to 22-38 eggs/g TS after dewatering. Inner composting temperatures were ≥45°C for 40 days. After co-composting for 110 days, the concentration was further reduced to 0.2-1.7 eggs/g TS. The viability of *Ascaris* eggs within the compost heap decreased from 58% initially to <10% after 60 days. The study also found that turning frequency had no impact on helminth egg reduction (220).

Cofie et al. (10) performed a pilot study for co-composting of solid waste and fecal sludge in Ghana. The treatment system included pre-treatment of fecal sludge through dewatering followed by co-composting of sludge with solid waste, and storage. Average fecal coliform and *Clostridium* concentration in dewatered sludge was 4.07 x 10<sup>8</sup> CFU/g and 4.93 x 10<sup>8</sup> CFU/g. Helminth egg concentration ranged from 25-83 eggs/g TS in the dewatered sludge. According to Feachem et al. (29), *Ascaris* eggs are inactivated after exposure to temperatures above 45°C for a minimum of five days. Therefore, while pathogen concentration was not measured as part of the study, the authors assumed the compost materials were sanitized since internal temperatures were above 45°C and composting occurred over 90 days (10).

El Fels et al. (221) conducted a pilot study of co-composting sewage sludge with date palm tree waste at 1:3 (Mixture A) and 1:1 ratios (Mixture B) for a period of six months. Initial concentrations of fecal coliforms and total coliforms were 12 x 10<sup>4</sup> CFU/100 mL and 345 x 10<sup>4</sup> CFU/100 mL for Mixture A, respectively, and 2 x 10<sup>4</sup> CFU/100 mL and 456 x 10<sup>4</sup> CFU/100 mL for Mixture B. After 180 days of co-composting, fecal coliforms and total coliforms concentrations decreased to ≤85 CFU/100 mL and ≤125 CFU/100 mL, respectively. Removal efficiencies were 99.9% and 99.5% for fecal coliforms in Mixture A and B, respectively and 99.99% for total coliforms in both mixtures (221).

A co-composting study in El Jadida, Morocco mixed sewer sludge with a sugar beet leaves (C1) and sewer sludge with a combination of sheep manure, sugar beet leaves, and straw (C2) (228). Co-composting of C1 and C2 was carried out in a bioreactor chamber for 30 and 23 days, respectively with temperatures >50°C for eight consecutive days. The process reduced total coliform

concentration from  $3.4 \times 10^9$  MPN/10g DS to 4.0 MPN/10g DS in C1 and from  $6.9 \times 10^7$  MPN/10g DS to 7.9 MPN/10g DS in C2. Fecal coliforms were reduced from  $6.8 \times 10^8$  MPN/10g DS to  $<1.3$  MPN/10g DS in C1 and from  $4.9 \times 10^7$  MPN/10g DS to 2.0 MPN/10g DS. For both mixtures, helminth eggs were reduced to  $<1$  egg/10g DS (228).

In a study from Egypt, composted sewage sludge was found to be free from *Salmonella* spp. and coliforms at the end composting at temperatures  $>50^\circ\text{C}$  for 12 weeks of fermentation and another four weeks of maturation (222). The study found coliform concentration to vary over the course of composting and decrease overall at a slower rate than the *Salmonella* concentration. A study from Porto Alegre, examined the pathogen reduction from co-composting of sewage sludge with solid waste (229). Temperatures ranged from  $37.0$ - $67.6^\circ\text{C}$ , with temperatures above  $45^\circ\text{C}$  for five consecutive days. At the end of 15 days, the compost mixture was free from detectable enteric viruses, *Salmonella* spp., helminth eggs, although the average *E. coli* concentration in the final compost ( $4 \times 10^4$  CFU/g) were above the WHO guidelines for agricultural reuse.

### Co-treatment with wastewater

In certain areas, fecal sludge is co-treated alongside wastewater. If the wastewater treatment facilities were not designed to handle fecal sludge, this could negatively impact the plant's ability to adequately treat either waste stream (78) and lead to overloaded tanks (230).

## 5.5 Sewers

Sewers are often considered the preferred sanitation technology and are most frequently used in developed countries (231). In sewerage systems, human excreta and urine are mixed with water and flushed through individual household connections to a piped network. Sewerage (also referred to as 'off-site sanitation') is more expensive than on-site sanitation systems (231). In addition, sewerage requires large volumes of readily available water, which is limiting for households without at-house water supplies and can elevate stress in water scarce regions. While wastewater is conveyed away from the household, there are numerous pathways whereby human excreta is unsafely leaked back into the environment in sewer systems.

### 5.5.1 Containment/Transport

Wastewater flushed from a toilet connected to a sewer system may leak unsafely back into the environment through misconnected sewer lines and/or breaks within the piping or sewer system. In the case of combined sewers, where sanitary sewers are connected with stormwater sewers, there is potential for wastewater to be discharged into the environment during periods of severe rainfall. Each of these potential pathways will be examined in detail in the following sections.

#### Misconnections

The term "misconnection" refers to a situation where a sanitary or greywater sewer pipe is connected to a surface water pipe unintentionally (25). Thus, wastewater is conveyed to surface water outfalls without any treatment. These may be accidental or intentional, illicit connections. Five studies on misconnections were included in the review.

Dunk et al. (25) undertook case studies of communities along the Thames River in the United Kingdom to determine the extent of misconnections. In total, the study examined almost 50,000 properties and found an average misconnection rate of 1.3% (range <1-9%). Toilet wastewater accounted for only 3% of misconnections with the majority (almost 70%) resulting from greywater appliances (e.g. sinks, hand basins, and washing machines) (25).

A study examining the microbial contamination of the Arkansas River found high concentrations of fecal coliforms (40-11,384 CFU/100 mL) in sections running through the city of Tulsa, while upstream and downstream concentrations were much lower (2-96 CFU/100 mL) (232). Raw sewage was suspected to be the main pollutant, therefore city officials examined sections of sewer lines near major storm drains. Upon inspection, a “T” connection in the sewage pipes was allowing sewage to flow directly into a stormwater drain (232).

Three studies were reviewed that examined illegal connections of sewage discharge into storm sewers. Johnson and Toumari (233) reported on a study of illegal connections to storm sewers in Wayne County, Michigan. Of the 3,340 businesses and industries examined, 9% had illicit connections, however only 11% (n=33) of those were from toilets. Li et al. (234) assessed the impact of illegal connections on stormwater sewers in Shanghai and Hefei, China. Illicit connection rates ranged from 27.1-51.7% in Shanghai, with lower rates observed in more recently built systems. Rates in some areas were as high as 90% in older systems of Hefei. Inceptors were built to try and reduce the volume of dry weather discharge from illegal connections, however the capacity of inceptors were not always sufficient to eliminate some wastewater being discharged into receiving waters (234). Another study in Shanghai used a water flow balance to estimate the extent of misconnections. They calculated 51% of collected sewage made its way into stormwater sewers (235).

### **Exfiltration**

Pipes may become compromised due to structural defects such as cracks, fractures, joint displacement, deformation and collapse, or due to operational damage such as roots, siltation, and blockage (26). Aging sewer pipes and infrastructure can allow sewage to leak out into surrounding soil and groundwater (exfiltration) or, in some cases, allow water to leak into the sewage system (infiltration). The position of sewer lines relative to the groundwater table determines whether exfiltration or infiltration occurs more often (236). Leaking sewer lines situated above groundwater will result in exfiltration, while sewer lines lower than groundwater will most likely experience infiltration (236).

Sewer pipe exfiltration can result in contamination of drinking water if drinking water pipes are nearby and have suffered similar structural or operational damage. However, exfiltration has a greater impact on groundwater quality in areas where the water table is shallow. Previous studies have indicated exfiltration of sewage pipes as the contamination source in groundwater supplies (237).

The extent of exfiltration is often difficult to estimate, but numerous direct and indirect methods have been developed. Rutsch et al. (238) conducted a review of different approaches to model and estimate exfiltration in sewers. The results of the reviewed studies and some additional studies found by this review are included in Table 18. The review concluded that there is large uncertainty in

the estimates of exfiltration rates, and the extent of exfiltration is still unclear in the body of available research. In addition, as is demonstrated in Table 18, there is a wide range of measurements used to report exfiltration estimates, which complicates comparability of results. Another review on exfiltration rates estimated 3-5% as an average exfiltration rate for pre-1960s sewers (239). In a study of urban recharge in Nablus, West Bank, Borst et al. (240) estimated 22% of wastewater is leached into the ground, with 57% being from sewer exfiltration and 43% from cesspit leakage.

**Table 18. Reported rates of exfiltration from sewer lines in the literature.** Adapted from (238).

Method used	Space and time scale	Country	Region	Extent of exfiltration (units vary)	Reference
Groundwater flow modelling, solute balances	Catchment (months-years)	England	Nottingham	5%, 6-13 mm/yr	(241)
		Austria	Linz	1% dwf <sup>a</sup>	(242) as cited in (238)
Groundwater sampling	Catchment (months-years)	Spain	Barcelona	2.4-89% of total recharge	(243) as cited in (238)
		---	---	8.64-38 l/m d	(244) as cited in (238)
		England	Doncaster	5-10%	(245)
Balancing time series	Catchment (months-years)	---	---	1.5-39 l/m d	(246) as cited in (238)
		Germany	Dresden	2.8% dwf	(247) as cited in (238)
	Pipe (weeks)	US	New Mexico	17-56%	(248) as cited in (238)
Balance of artificial tracer load	Pipe (minutes)	Switzerland	Rümlang	11% dwf ( $\pm 2\%$ )	(249) as cited in (238)
		Germany	Berlin and Dresden	7-167 l/m d ( $\pm \max 13\%$ )	(250) as cited in (238)
		Italy	Rome	12.8% dwf (agricultural areas) 20.8% dwf (urban areas)	(251)
Pressure tests	Pipe/damage (minutes)	---	---	1.6-58 l/day cm <sup>2</sup>	(252) as cited in (238)
		---	---	0.07-109 l/day cm <sup>2</sup>	(253) as cited in (238)
Volumetric measurement	Pipe/damage (minutes)	---	---		(254) as cited in (238)
Circling waste water through leaky pipe in soil bedding	Laboratory (minutes-weeks)	---	---	0.87-5.2 l/d cm <sup>2</sup>	(255) as cited in (238)
		Denmark	Frejlev	0.02-0.06 l/d cm <sup>2</sup>	(256) as cited in (238)
		---	---	9-86 l/d cm <sup>2</sup>	(257) as cited in (238)
Integral pumping tests	Wells (Days)	Germany	Leipzig	28.0-63.9 L/m d	(258)

<sup>a</sup>dry weather flow

### Combined sewer overflows

In certain areas, combined sewers connect sewerage systems to stormwater pipes. During periods of intense rainfall, these systems allow wastewater and stormwater to bypass WWTPs, resulting in a discharge of untreated wastewater. These events are referred to as “combined sewer overflows,” or CSOs. According to the U.S. EPA, in 2004 there were 9,348 CSO outfalls in the United States, with most of them concentrated in the Great Lakes region (259). In the same report, the authors estimated the volume of untreated CSO discharge to be 850 billion gallons/year. Combined sewers have been built in the U.S., Canada, Europe and parts of Asia.

Similarly, sanitary sewer overflows (SSOs) may also occur due to clogged or broken pipes, infiltration, or power failures. While not all SSO events are reported, the U.S. EPA are an estimated 23,000-75,000 SSO events per year (259). Of the events reported during a three year period, over 80% consisted of less than 10,000 gallons. In the U.S., CSOs and SSOs are the main source of human fecal contamination in surface water (260). One study reported an estimated 39 million m<sup>3</sup> of untreated sewage and stormwater is discharged into the River Thames in London during 60 overflow events from 34 different CSO outfalls.

During a CSO or SSO event, large volumes of untreated wastewater may be discharged into surface water bodies. The EPA reported a range of 10<sup>5</sup>-10<sup>7</sup> fecal coliform colonies/100 mL in untreated wastewater and a range of 3-40<sup>6</sup> fecal coliform colonies/100 mL in CSOs (259). The bacterial load fraction in surface water bodies due to CSO events were estimated to be 10-15% in the Michigan Rouge River and up to 61% in the Anacostia River in Washington, D.C (259).

Data on frequency of CSO events in Shanghai, China and Japan were cited in Li et al. (261). In 2005, 80% of combined sewers in Shanghai overflows nine times or less, while 20% overflowed between 10-19 times. However the frequency of overflows was higher in 2004, with 55% overflowing nine times or less, 40% overflowed 10-19 times, and 5% overflowed 20-29 times during the year. In contrast, from data of 192 combined sewers in Japan, 5% overflowed nine times or less, 10% between 10-19 times, 16% between 20-29 times, and 69% over 30 times (up to 69 times as a maximum) (261). The frequency of CSO events depends on multiple factors including precipitation and sewer characteristics.

Numerous studies reported high pathogen loads in receiving waters from CSO events. One study estimated the discharge load from a CSO to contain 79 and 100 times more *E. coli* and intestinal enterococci, respectively, than the combined total median outfalls from three WWTPs (27). Ham et al. (262) estimated CSO outfall and stormwater accounted for 4-23% of the total indicator bacteria concentration in the Tama River in Tokyo. Another study calculated distances of 3.5 km and 2.1 km were needed for a 90% reduction in fecal coliform and fecal streptococci concentration from a CSO outfall in north London (263). Fecal coliforms and fecal streptococci have been observed to multiple and survive in sediments from receiving waters (263).

In contrast, McLellan and Salmore (264) monitored *E. coli* concentration in the South Shore Marina in Milwaukee during two CSO events. They found no statistical significant difference in the geometric means of *E. coli* concentration between the two events, even though one discharged over 100 times more volume of wastewater (264). The study found the CSO events did not increase contamination

levels on the shoreline, but found high *E. coli* concentrations in the beach area regardless of CSO events.

With regards to protozoan pathogens, three studies examined *Giardia* concentrations in CSO discharges. In Germany, a study of CSOs found a two log increase in total coliform and *Giardia* concentration in a river body during a CSO compared to dry weather conditions (265). Annual *Giardia* loading from CSO events were substantially higher ( $1.2 \times 10^{10}$  cysts/yr) than loading from sewage treatment effluent ( $3.0 \times 10^9$  cysts/yr) (265). Results of the study showed periods of heavy rainfall led to higher levels of pathogens than longer periods of moderate precipitation (265).

In 1998, Gibson III et al. (266) studied protozoa concentrations in the Saw Mill Run stream, which received 26 CSO outfalls. During dry weather conditions, concentrations of *Cryptosporidium* and *Giardia* ranged from 5-105 oocysts/100 L and 13-6,579 cysts/100 L, respectively (266). However during five CSO events, concentrations ranged from 250-40,000 oocysts/100 L and 9,000-283,000 cysts/100 L. A more recent study conducted microbial risk assessment of CSOs along the Lower Passaic River in New York (267). During one CSO event, an estimated 125 million gallons of sanitary sewage and stormwater are released into the river. Samples from the CSO outlet contained >30,000 CFU/100 mL of total coliforms, fecal coliforms, fecal streptococcus and fecal enterococcus (combined). *Giardia* concentration ranged from 1,860 cysts/L at the outfall to 798 cysts/L ten feet downstream, and *Cryptosporidium* was not detected (267).

Two studies examining concentrations of viruses in CSO outfalls were reviews. Fong et al. (268) found average adenovirus DNA concentrations from six CSO events to be  $5.35 \times 10^5$  viruses/L, which was a little less than half the concentration found in raw sewage ( $1.15 \times 10^6$  viruses/L). A study of a CSO next to a WWTP in Tokyo found a one log decrease in total coliform concentration in receiving waters the day after a CSO event (269). However, in the receiving waters of the CSO, the proportion of samples positive for viruses remained relatively constant four days after the event. The study was unable to confirm if this was due to viruses persisting in the environment or due to ineffective treatment from the WWTP (269).

## 5.5.2 Treatment

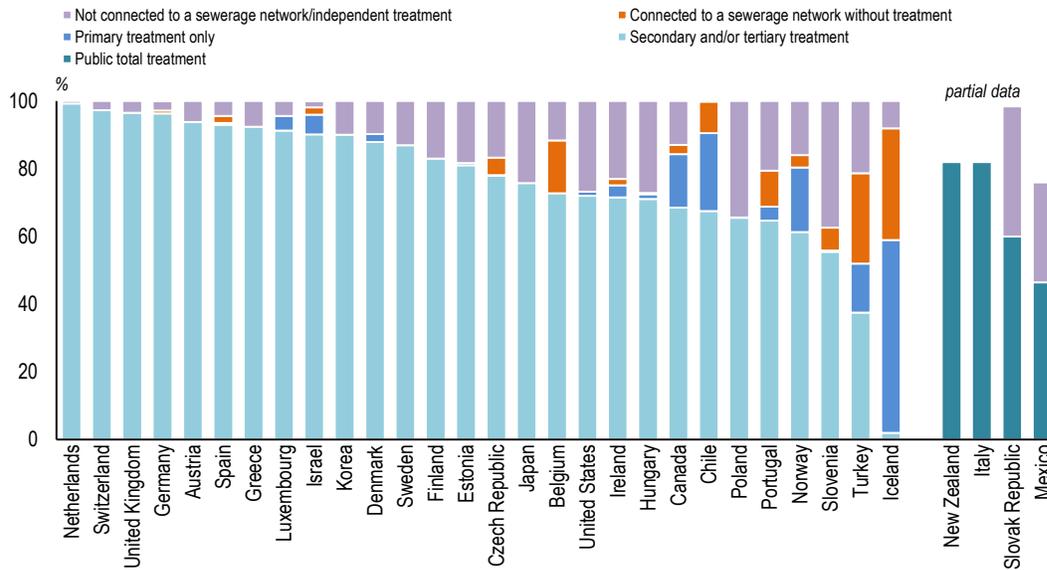
### *Available treatment facilities*

The 2000 Global Water and Sanitation Assessment from the World Health Organization and UNICEF reported estimates for the population in large cities with access to sewers and the percentage of wastewater treated using at least secondary treatment processes. North America had the highest percentage of the population in large cities using sewers (96%) as well as the highest percentage of wastewater undergoing at least secondary treatment (90%) (270). According to the report, only 18% of the population in large cities in Africa were connected to sewers and 0% of wastewater was treated using secondary treatment (270). While 92% of the population in large cities in Europe were connected to sewers, only 66% of wastewater received secondary treatment (270). The WHO/UNICEF report illustrates that globally, a small percentage of wastewater undergoes secondary treatment processes.

A more recent study by Malik et al. (271) developed a global database of sewage connection rates and wastewater treatment rates. The study drew on data from the *Present Masons Water Yearbook*, United Nations Statistics Division, Organisation for Economic Cooperation and Development, and the Food and Agriculture Organization of the United Nations as well as additional publicly available data. The study identified data for 183 countries and normalized wastewater treatment rates by multiplying it by the national sewage connection rate to calculate a wastewater treatment indicator. Connection rate and treatment level were positively correlated, however there were outlier countries with low connection rates and high treatment levels (Thailand, Cape Verde, Palau, and American Samoa) and countries with high connection rates and low treatment levels (Maldives, Colombia, and Georgia). Regions where connection rates were greater than treatment rates by 10% or more included Eastern Europe and Central Asia, Latin American and the Caribbean, the Middle East and North Africa, North America, and South Africa. The available data reveals that even in countries where households are connected to sewerage, there may be large percentages of wastewater that is discharged untreated.

The study authors reported several challenges on locating and comparing data on wastewater treatment. Since data sources rarely distinguished wastewater sources, wastewater data included domestic, commercial, industrial, and stormwater runoff from urban areas. Similarly, the study reported most data sources did not contain disaggregated data on the level of wastewater treatment (primary, secondary, and tertiary), therefore treatment was considered primary and above (271). Additionally, the study authors noted that recent data (post 2005) was unavailable for 96 countries.

Two datasets that contained national data on the extent of wastewater treatment coverage or type of treatment processes were obtained, however the data was mostly limited to European or developed countries. The Eurostat website provides publicly available data on the level of treatment and the type of treatment from countries within the European Union. Data on the level on treatment in 2013 for some countries was available on the OECD website (Figure 2). Figure 2 demonstrates that even in developed countries, domestic wastewater from sewered households is discharged without treatment (272). Few studies or reports were identified that detailed the level of wastewater treatment in low, lower-middle, and upper-middle income countries.



**Figure 2. Percentage of the national population connected to a wastewater treatment plant by level of treatment.** From (272).

Several studies indicated that large volumes of wastewater were discharged without treatment in major cities. Table 19 contains reported percentages of wastewater discharged untreated into the environment in various areas. While only 15% of Accra is served by a central sewage system, according to Murray et al. (76), all of the wastewater collected by the sewer system is discharged directly into the ocean without treatment. Similarly, while Tema has a wastewater treatment facility, it is no longer functioning and all wastewater is now discharged into the ocean (76).

Direct discharge of wastewater into surface water can result in high levels of microbial contamination. Yapo et al. conducted a quantitative microbial risk assessment (QMRA) of a wastewater canal and lagoon located in Abidjan, Côte d'Ivoire. Domestic and industrial wastewater was discharged into drains that fed directly into the canal, and the study authors found concentrations of *E. coli* ranging from 12.8- 2.97 x 10<sup>4</sup> CFU/100 mL and *Giardia lamblia* ranging from 0-18.5 cysts/L (273).

**Table 19. Reported percentage of untreated wastewater and location of discharge.**

Country	Region/City	% discharged untreated	Location of discharge	Reference
Ghana	Tema	100%	Ocean	(76)
India	Bihar and West Bengal	83.7%	Surface water bodies	(274)
		5.7%	Impounded earthen ponds	
	10.6%	Reused for agricultural purposes		
India	Large cities	80%	Natural water bodies	(275) as cited in (76)
Indonesia	Banjarmasin City, Malang City, Payakumbuh City (urban)	65% 8%	Leached into ground Waterways	(276)
	Lamongan District, Tangerang District (rural)	86% 8%	Leached into ground Waterways	
Nepal	Kathmandu Valley	95%	Rivers	(13)
	Kirtipur	100%	Rivers	(80)
Palestine	Nablus	21.7%	Leached into ground	(240)
		78%	Valleys	
Senegal	Dakar	22% <sup>a</sup>	Ocean	(15)
Thailand	Klong Lang Municipality	90%	Waterways	(81)

<sup>a</sup>Percentage calculated by the amount of wastewater discharged untreated divided by the total volume of wastewater received by WWTP

Similar to FSTPs, if WWTPs are available, they are often operating under capacity or are incapable of treating the total volume of generated wastewater (96, 254). In Accra, although there are 31 WWTPs, even if all were functional they would only serve a fraction (<10%) of the population (82). A 2012 report by the Department of Water Affairs in South Africa found that 317 WWTPs required urgent attention, 143 had a high risk of failure, and 20% were operating over capacity (278). In Egypt, some treatment facilities in major urban areas like Cairo and Luxor are operating close to their designed capacity, while others are operating at 2-62% of their designed capacity (98). In Delhi, India, Jamwal et al. (279) estimated daily wastewater production rate of  $2.87 \times 10^6$  m<sup>3</sup>/day, although the total combined designed capacity of the 17 sewage treatment plants was only  $2.30 \times 10^6$  m<sup>3</sup>/day. In actuality, the shortfall in treatment capacity was greater considering certain treatment facilities were out of operation, which led to only 40% of generated wastewater being treated (279).

An assessment of wastewater management technologies in the Caribbean Region acknowledged that lack of technical knowledge, insufficient funding, poor operational and maintenance practices, and inappropriate technology were factors leading to the failure of many WWTPs (280). Similar factors contributing to the failure of treatment facilities were found in Ghana. These included lack of trained operators, lack of funding for preventive maintenance, and inconsistent electricity (82). Of the 71 treatment plants (both wastewater and fecal sludge treatment plants) identified in Ghana, 21 were operating with at least one component failing (82). The total included both public and privately operated facilities, with only 16% serving a municipal or town, 25% serving a community, 20% serving

businesses, 13% serving schools, 13% serving military camps, and 13% serving hospitals (82). Thirty-five of the plants were found to be non-functional and six were of unknown status (82). The ability of wastewater treatment facilities to monitor effluent is also a concern. Some plants do not have the proper equipment to test the quality of effluent before discharging (98).

Certain cities had more data available, however reported figures regarding the volume of wastewater treated varied across the literature. According to Dodane et al. (281), the Cambèrène WWTP in Dakar, Senegal uses two settling tanks (19,200 m<sup>3</sup>/day combined capacity) for primary treatment and activated sludge as secondary treatment (9,600 m<sup>3</sup>/day). Sludge is then treated using anaerobic digesters and then disposed of through land application. A fraction of secondary effluent (5,700 m<sup>3</sup>/day) is further treated through sand filters and disinfection, then used for irrigation purposes. The remaining secondary effluent is discharged to the ocean (281). However a different paper claimed only 15% of wastewater generated in Dakar was treated using activated sludge processing and the remainder was discharged untreated into the ocean (90). Yet another report found 47% of surveyed households in Senegal discharged their wastewater directly into the streets and 9% discharged into open canals (15).

### *Treatment efficiencies*

In areas where wastewater treatment facilities are available, there is still potential for unsafe return of human excreta depending on the efficiency of the treatment process to remove harmful pathogens. Table 20 summarizes relative efficiencies for bacteria, virus, protozoa, and helminth removal of various treatment processes reported in the literature. If the concentration of pathogens in influent wastewaters are high, then even multiple log reductions may not reflect the pathogen concentration in the effluent.

**Table 20. Relative efficiencies of pathogen removal of sewage treatment operations and processes.**

Treatment level	Specific treatment process	Bacteria removal (%)	Virus removal (%)	Protozoa removal (%)	Helminth removal (%)	Reference
Primary	Fine screening	10-20				(282)
Disinfection	Chlorination of raw/settled sewage	90-95				(282)
Primary	Plain sedimentation	25-75 50-90	0-50 0	0-50	50-70 <sup>a</sup>	(282) (29) (283) (33) (284)
Primary	Chemical precipitation	40-80 90	90	50-80	90-99	(282) (283) (33)
Primary	Trickling filtration	90-95 <sup>b</sup> 80-99 <sup>a</sup> 0-99	15-75 50 0-90	20-90 <sup>1</sup> 0-90	20-90 <sup>a</sup> 94-100 <sup>b</sup> 0-90	(282) (285) (283) (284)
Secondary	Membrane bioreactors	Up to 99.9999	Up to 99.9999	Up to 99.9999	Up to 99.9999	(33)
Secondary	Activated sludge treatment	90-98 <sup>c</sup> 60-99 99-99.9	Up to 99 90 90-99	90-99	80-100 70-90 90-99	(282) (285) (283) (33) (284)
Secondary	Stabilization ponds	95-98 Up to 99.9999 99.9-99.9999 99.99-99.9999	Up to 99.99 99-99.99 99-99.99	99-100 90-99 99.99-99.9999	100 99.9 99.99-99.9999	(282) (286) (33) (284)
Disinfection	Chlorination of biologically treated sewage	98-99 99	90-99.9	0-96.8	0	(282) (33)
Disinfection	Ozone	99 <sup>d</sup>	99.9 <sup>d</sup>	≥99 <sup>d</sup>		(283)
Secondary/	Constructed	90-98		60-100		(33)

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Tertiary wetlands

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<sup>a</sup>typical removal rates reported

<sup>b</sup>followed by plain sedimentation

<sup>c</sup>preceded and followed by plain sedimentation

<sup>d</sup>at specific ozone residuals, contact times, and temperatures

### **Trickling filters**

Robertson et al. (287) conducted a three year study on the protozoan removal efficiency of six WWTPs in the United Kingdom. Two of the plants used primary sedimentation followed by secondary treatment using a trickling filter. One of the plants applied additional tertiary treatment through filtration. Removal efficiencies for *Giardia* and ranged from 74% (SD 15) to 85% (SD 30). The removal efficiencies for *Cryptosporidium* were much lower, ranging from 5% (SD 35) to 38% (SD 38%) (287). These were within the ranges reported by Feachem et al. (29) and (284) for trickling filters protozoa removal rates.

### **Activated sludge**

Six studies of wastewater treatment facilities using activated sludge processes were reviewed (171,287–291). Two log removals of fecal coliforms were demonstrated in both studies measuring bacterial concentration. Removal efficiency for protozoa ranged from 28% to 100%, while enterovirus removal was 98%, however only one study measured virus concentrations (288). Helminth removal also varied from 67% to <99.6%. Table 21 summarizes the removal efficiencies of pathogens from the four studies.

**Table 21. Reported treatment efficiencies for activated sludge processes in literature.**

Country	Treatment facilities (N)	Fecal coliforms	Coliphage	Enteroviruses	<i>Giardia</i>	<i>Crypto-sporidium</i>	<i>Ascaris lumbricoides</i>	<i>Entamoeba histolytica</i>	Reference
US	1	99.1%	82.1%	98.0%	93.0%	92.8%	<99.6%		(288)
UK	3				66-94%	28-98%			(287)
Canada	3	98.7%-99%	96.8-99.5%						(171)
Jordan	1				100%		88%	67%	(289)
Brazil	8	99%							(290)
Iran	1				96.7%		97.4%		(291)

Kerstens et al. (292) evaluated three different types of decentralized wastewater treatment facilities operating in Java, Indonesia. System 1 consists of a settler, anaerobic baffled reactor, and an anaerobic filter. System 2 treats blackwater through a digester, then combines treatment with grey water in a settler, anaerobic baffled reactor, and an anaerobic filter. The final system uses a settler, equalization activated sludge, clarifier and filtration. The effluent from nine sites (5 sites using System 1, 3 sites using System 2, and one site using System 3) were tested for various water quality parameters. The total coliform concentration from the different systems ranged from  $1 \times 10^8 - 1 \times 10^{18}$  CFU/100 mL,  $1 \times 10^7 - 1 \times 10^{17}$  CFU/100 mL, and  $1.5 \times 10^5 - 1.5 \times 10^{10}$  (estimated from graphs). The system utilizing an activated sludge process had the lowest total coliform concentrations, however this was based on only one sampling site (292).

### **Waste stabilization ponds**

Waste stabilization ponds (WSP) are classified into three different types: anaerobic, facultative, and maturation (286). Anaerobic and facultative ponds are used to reduce the biological oxygen demand, while subsequent maturation ponds are used to reduce the concentration of fecal bacteria present in the wastewater (286). Table 6 in Appendix A contains various pathogen removal efficiencies reported in literature for different WSP systems. Heinss et al. (213) states that there is a one log reduction in fecal coliforms for each anaerobic pond in a series, and two maturation ponds in series can result in one log reduction. Depending on the WSP configuration, one to six log reductions in fecal coliforms have been demonstrated. Virus removal rates were between 90-99.9%, with greater reductions occurring in systems with additional maturation ponds (213).

According to studies conducted by Ayres et al. (293) in Brazil, India, and Kenya, there is no statistically significant difference in the efficiency of helminth egg removal between anaerobic, facultative, and maturation ponds. Ayres et al. (293) developed the following equation [1] to calculate percentage removal of helminth eggs based on retention time, where R is the percent removal in anaerobic, facultative, or a maturation pond and  $\theta$  is retention time in days:

$$R = 100[1 - 0.14e^{-0.38\theta}] \quad [1]$$

Mara (286) suggests if the effluent from a facultative pond contains >1 helminth egg/liter then one or more maturation pond(s) should be included after the facultative pond to ensure proper helminth removal. If facultative pond effluent contains <1000 CFU/100 mL *E. coli*, then helminth egg concentrations are likely to be less than <<1 egg/L (286). Studies have shown that 100% removal rates of helminth eggs can be achieved in various WSP schemes with a 10-14 day retention time (294). In the four pond stabilization scheme, Heinss et al. (213) recorded helminth eggs in waste pond effluent with retention times of 9, 4, 4, and 4, respectively in the four ponds. For anaerobic or facultative ponds, complete helminth egg removal has been reported after two to three weeks (213). Once helminth eggs settle in pond sludge, they may remain viable for long periods of time, as one study found viable eggs in sludge stored for nine years (295).

Ingallinella et al. (296) piloted co-treatment of septage and sewage in a wastewater stabilization pond system. Septage was discharged into a primary pond, then combined with sewage and treated in two ponds in series. One primary pond was used from January to July of 1999 (Phase I), however

sludge accumulated to the point where a new primary pond (of the same dimension) had to be used from July 1999 until February 2000 (Phase II). Fecal coliforms were reduced from  $1.73 \times 10^7$  MPN/100 mL in the septage effluent to  $1.06 \times 10^5$  MPN/100 mL in Phase I and from  $6.0 \times 10^6$  to  $1.2 \times 10^5$  MPN/100 mL in Phase II. The helminth concentration within the sludge increased from 5,500 eggs/100 g dry weight in May 1999 to 6,000 eggs/100 g dry weight in April 2000. While the study demonstrated the ability of WSP systems to reduce fecal coliforms, however the results emphasize the potential of helminth contamination increasing in WSP sludge (296).

The siting of WSPs should take into account the depth of groundwater as large volumes of effluent will be pooled in one location. Knappett et al. (297) examined the microbial contamination of a shallow aquifer located beneath a wastewater ponds in Bangladesh. The results indicated latrine effluent receiving ponds can be sources of significant bacterial contamination. Sediment grain size, pond age, and seasonality affected the extent of bacterial contamination with newer ponds and monsoon conditions resulting in the highest bacterial concentrations. The authors suggested a minimum lateral distance of 13 meters between wells and ponds given the conditions within the study setting, however the distance may need to be greater in areas with larger grain-sizes (297).

#### Disinfection processes

Hendricks and Pool (298) studied three WWTP in South Africa that used the same primary and secondary treatment processes (screens as a pre-treatment and activated sludge treatment) but each used a different tertiary process. Of the three WWTP, one used ultraviolet (UV) light as a disinfection step, while another used chlorination, and the last WWTP did not use a disinfection step but rather a membrane bioreactor. In comparing the treatment efficiencies of the three plants, the effluent from the WWTP using UV light contained  $>1000$  CFU/100 mL of detectable *E. coli*, while the effluent from the other two WWTP using chlorination and a membrane bioreactor contained  $<1$  CFU/100 mL (298).

#### Specific case studies of treatment efficiencies

A recent study examining water quality of the Upper Santa Cruz Watershed in Arizona measured the microbial concentration of effluent discharged from three WWTPs (299). The type of treatment processes used at the plants were not specified in the study. Arizona state regulations require for any given sample not to exceed 235 *E. coli* CFU/100 mL or 800 CFU/100 mL of fecal coliforms. The study reviewed monthly discharge reports from three WWTPs from 1998-2008 for fecal coliform measurements and from 2008 to 2011 for *E. coli* measurements. Of the monthly reports reviewed, 13-15% exceeded the maximum concentration for fecal coliforms and 16-34% exceeded the maximum *E. coli* concentration. The study demonstrates that although municipal wastewater was treated prior to discharge, the concentration of pathogens still exceeded state water quality limits (299).

Gennaccaro et al. (300) examined the presence of *Cryptosporidium parvum* at six reclamation facilities in the US over the course of five to twelve months. The various treatment plants used different filtration processes (e.g. shallow or deep-bed sand and anthracite filters or fabric disk filters) and disinfection (chlorine or UV). Infectious *Cryptosporidium* oocysts were detected at each stage in the treatment process with 40% of final effluent samples being positive for infectious oocysts (Appendix A, Table 7) (300).

Rose et al. (288) conducted a year-long study of a wastewater reclamation plant in Florida to evaluate the treatment efficiency for the removal of various pathogens. The reclamation facility processed 16 million gallons per day using preliminary screening, biological treatment with activated sludge, dual-media rapid sand filtration with in-line addition of alum and polymer coagulants. After filtration, effluent was chlorinated and stored in storage tank for 16-24 hours before being discharged. The resulting sludge was then treated using thickening, anaerobic digestion, and dewatering (288).

Complete treatment of wastewater as described above yielded high reductions (>99.999%) of total coliforms, fecal coliforms, and phages. Enteroviruses, *Giardia*, and *Cryptosporidium* removal rates were 99.999%, 99.993%, and 99.95% respectively. Complete sludge treatment resulted in 92%, 90%, 83% and 68% removal rates for total coliforms, fecal coliforms, coliphages, and enteroviruses, respectively. However, the final concentration of fecal coliforms was still high (geometric mean  $2.4 \times 10^5$  CFU/g). Removal rates were >99% and 97% for *Cryptosporidium* and *Giardia*, respectively, while there was complete removal of helminths (288) (Additional data from the study is available in Appendix A, Tables 8 and 9).

A comparison study of centralized and decentralized treatment processes in Sydney, Australia measured the concentration of *Cryptosporidium* oocysts and enteric viruses in effluent (301). They found *Cryptosporidium* oocysts in 76% of samples from sewage WWTP effluent (median 0.7 oocysts/L, max 290 oocysts/L) and in only 8% of septic tank effluent (range 230-510,000 oocysts/L). Enteric viruses were present in 17% and 50% of effluent samples from sewage WWTPs and decentralized sewage systems, respectively. The type of treatment processes used by the sewage treatment plants were not specified (301).

Lim et al. (302) examined the efficiency of two community wastewater treatment systems in Indonesia. The first system used biological contact filters and surface aeration while the second system uses a rotating biological contactor process. The *E. coli* concentrations in the effluent from the two systems ranged from  $5.4 \times 10^6$  MPN/100 mL (dry season) to  $1.6 \times 10^9$  MPN/100 mL (wet season) and  $2.4 \times 10^6$  MPN/100 mL (dry season) to  $5.4 \times 10^7$  MPN/100 mL (wet season), respectively. No data was reported on the treatment efficiency of either system, however, the effluent still contained high concentrations of fecal indicator bacteria. While Indonesia has wastewater effluent standards, there is no specific limit for *E. coli*. The effluent was discharged into irrigation canals that eventually flow into rice paddies. The *E. coli* concentration in the irrigation canals downstream of the WWTP ranged from  $1.7 \times 10^5$  –  $1.6 \times 10^7$  MPN/100 mL (302).

### 5.5.3 Disposal

#### *Disposal locations of treated wastewater*

A study of 138 wastewater treatment facilities throughout the Caribbean region reported 29% of plants discharged into marine environments, 22% discharged into freshwater, 14% discharged into the sub-surface, and 14% discharged on-site (storm drains, street drains, gullies, and fields) (212). They found the effluent from 21% of surveyed facilities was reused in irrigation or flush water for toilets (212). In the Gaza strip, effluent from two WWTPs are discharged into the sea as well as 18 pipelines

conveying wastewater (303). In Beijing, China 400 Mm<sup>3</sup> of municipal wastewater is treated using tertiary or advanced treatment and then reused (76).

The practice of using wastewater for agricultural purposes is expected to grow as the allocation of freshwater supplies for agriculture is reduced due to growing domestic and industrial water demands (304). Wastewater reuse can be classified into direct use of treated wastewater, direct use of untreated wastewater, indirect use of untreated wastewater, and planned wastewater reuse (305). The use of untreated wastewater in agriculture may be planned or unplanned, as farmers may be unaware that they are using discharged wastewater that has not undergone treatment (305). The application of untreated wastewater in agriculture presents another potential pathway for the unsafe return of human excreta to the environment. Data on the volume of untreated wastewater used directly for irrigation purposes was only available for certain countries from the Food and Agriculture Organization of the United Nations (FAO) Aquastat website (Table 22). While the latest available figures are reported in Table 22, the data for some countries is outdated (India and China).

**Table 22. Volume of untreated wastewater directly used for irrigation purposes in various countries.** From (306).

Country	Year	Volume of untreated wastewater used directly for irrigation purposes (10 <sup>7</sup> m <sup>3</sup> /yr) <sup>a</sup>
Bolivia	2008	1.57
China	1995	0.1
India	1985	123
Iran	2005	24.4
Iraq	2012	103
Mexico	2004	433
Morocco	2010	1.2
Pakistan	2006	102.2
Syrian Arab Republic	2009	41.6
Tunisia	2008	4.66

<sup>a</sup>this includes wastewater used for landscaping and forestry.

Data on the disposal of fecal sludge produced from WWTPs were mostly found for European countries. A recent study by Kelessidis and Statsinakis (307) examined the sewage sludge treatment and disposal practices of countries within the European Union. Amongst the 27 member nations of the European Union (EU), an average of 10,957 x 10<sup>3</sup> ton dry solid/year of sewage sludge is produced (307). Of the 27 member countries, 11 have limits for pathogen concentrations in disposed sludge (307). While the study reported that a variety of sludge treatment methods are used across Europe, anaerobic and aerobic digestion are the most common forms of treatment. A full table of treatment methods used in EU member states is available in Appendix A, Table 10.

The most common methods of disposal across member nations are agricultural use (41%), incineration (19%), landfilling (17%), composting (12%), and “others” (12%) (307). In 1998, the EU banned the dumping of sewage sludge into the ocean, thus the data reported 0% of disposal methods included the disposal of sludge to surface water in 2000 (307). However, the study speculated that countries may continue the practice but report the method under the “other” category (307). Some countries reported high “other” percentages and did not specify specific

treatment methods, leaving uncertainty as to the final fate of sewage sludge (307). The study noticed regional patterns as well as patterns between “old” member nations and “new” member nations, with older members practicing less landfilling (15%) compared to newer members (28%), and more agricultural use (44% vs. 16%) and incineration (21% vs. 1%) (307). The study noted that there may be some discrepancy in the data between countries as the definitions for composting and agricultural use are not clearly defined and some countries may report one within the other category (307).

Two studies of WWTP sludge disposal practices in Latin America were reviewed. Vlugman (212) surveyed 138 of the 303 existing WWTPs in the Caribbean region. Of the surveyed facilities, 23% did not remove sludge from the system, 25% applied sludge to land, 14% disposed of sludge at landfills, and 9% injected sludge into deep wells. Two facilities discharged sludge on-site, while one disposed of sludge in storm drains and another into coastal waters. In some areas of Argentina, septage sludge from wastewater treatment facilities is delivered to farmers to be used as fertilizer (296). According to Argentine law, sanitary landfills are legal disposal sites for fecal sludge, if the sludge and other waste are contained in separate cells (296).

#### Extent of treatment for safe return

The pathogen concentration deemed “safe” for the disposal of wastewater and human excreta depends on its ultimate purpose or fate. In certain areas of the world, the reuse of human excreta for fertilizer is a long standing practice. Similarly, many countries reuse wastewater for irrigation purposes, especially in water scarce regions. In 2006, the WHO published guidelines for the safe application of wastewater and human excreta in agriculture and aquaculture (Table 23) (38,308).

**Table 23. WHO guidelines for the safe reuse of fecal sludge and wastewaters.** From (38,308).

	Helminth eggs (No/g TS <sup>a</sup> or No/L)	<i>E. coli</i> (No/100mL)
Treated feces and fecal sludge	<1/g TS	<1000/g TS
Wastewater for use in restricted irrigation (all crops not eaten raw)	≤1/L	≤10 <sup>3</sup>
Wastewater for use in unrestricted irrigation of crops eaten raw	≤1/L	≤10 <sup>3</sup>
Wastewater for use in aquaculture	ND/L	≤10 <sup>3</sup>

<sup>a</sup>gram of total solids

The question still remains as to what defines a “safe” return for waste disposed at disposal sites, surface water, or coastal areas. In areas where fecal sludge may be disposed of in an isolated area, it may be that higher initial levels of contamination are acceptable as pathogen-die off could occur without human exposure. Where wastewater or sludge is discharged into surface waters, there is concern for human exposure if communities are downstream of the discharge. Mara et al. (286) recommends river waters to contain ≤1000 *E. coli*/100 mL in areas where surface water is used for domestic purposes.

With regards to coastal waters, there is concern for public health in areas where people regularly swim and in areas where seafood is harvested. The Council of the European Communities (309) recommends bathing water to contain  $\leq 2000$  *E. coli*/100 mL and areas of commercial shellfisheries to contain  $\leq 10$  *E. coli*/100 mL. However, the Aruba Protocol limits the concentration of *E. coli* in marine waters to  $\leq 200$  *E. coli*/100 mL in areas with fragile marine life such as coral reefs, seagrasses, and mangroves (310). According to Feachem et al. (29), bacterial and viral survival times decrease in seawater, however protozoa and helminth survival times remain relatively the same.

In 1986, the US Environmental Protection Agency provided guidance for the development of microbial standards for recreational and marine waters (311). For recreational waters, where there is full body contact, the geometric mean of a statistically representative sample size ( $\geq 5$  samples over 30 days) should not exceed 126 CFU *E. coli*/100 mL or 33 CFU *Enterococci*/mL. However, within the US, individual states set their own water quality standards for recreational water.

## 6. Discussion

While latrines provide a relatively low-cost, low maintenance sanitation solution, this review found numerous factors that should be considered to ensure latrines properly contain human excreta. Not all latrines observed within the reviewed studies had a slab as part of their design, however the presence of a slab has been shown to reduce *E. coli* contamination in the latrine vicinity (312). The results and recommendations of the reviewed studies on pit latrine and groundwater quality were varied. Four studies concluded pit latrines did not pose a risk to groundwater. In contrast, 17 studies found a relationship between pit latrines and microbial contamination of groundwater.

Recommendations on reducing the impact of pit latrines on groundwater varied among studies included in the review by Graham and Polizzotto (18). Some suggested pit liners, others recommended elevating latrines, and others advised against constructing pit latrines in certain soil types (18).

In areas with heavy rainfall and frequent flooding, additional considerations are needed to ensure latrines are not compromised and fecal waste is safely contained. From the reviewed studies, it is unclear the fraction of waste that is dislodged from pit latrines during a flooding event. UDDTs may be a suitable alternative to pit latrines in flood prone areas, since they are constructed above ground and have a water-tight chamber for excreta (46). However, in some regions the cost of elevated toilets may be prohibitive for households (46). During flooding events, sanitation facilities may be compromised and households could be left without appropriate sanitation alternatives, resulting in open defecation and potentially leading to health outbreaks (313,314). Therefore choosing resilient sanitation technologies and restoring impaired facilities quickly after a flood event are important to reducing the risk of additional fecal contamination.

The percentage of households that emptied their latrine varied widely across reviewed studies, as one study reported all households emptied their latrine (15) and another study reported only 1.9% of surveyed households had emptied their latrine (64). The methods reported by households for pit emptying were different amongst the reviewed studies. Access to services, technological limitations,

and cost were found to be barriers to households using hygienic latrine emptying methods. Education about the role of pit latrines in containing human excreta may also serve to reduce unsafe return of human excreta through household behaviors such as the ‘flooding out’ of latrines. As one study highlighted the lack of knowledge about pit emptying services amongst households (41), education and social marketing programs could help raise awareness of the availability of these services.

The Philippines and South Africa implemented scheduled emptying services (78,315). However, the variability of filling rates for on-site sanitation facilities results in different timelines for household demand for emptying services (316). In the case of Durban, South Africa, the municipality found the practice too resource intensive and in some areas, pit latrines were inappropriate due to soil and groundwater depth (317). Therefore the municipality proposed an alternative sanitation technology, a double vaulted UDDT that can be manually emptied after a year of residence time (317). In the case of the Philippines, a scheduled emptying program has been more effective since most households have septic tanks that are well designed. This enables a more accurate estimation of filling rates (78).

Reviewed studies on FSM were only from a handful of developing countries, mainly from West Africa and Southeast Asia. The current JMP framework monitors household access to ‘improved’ sanitation based on the type of sanitation facility and does not consider if the entire sanitation delivery chain is in place beyond containment. However if the safe return of human excreta is added to the definition of ‘improved sanitation’, it is estimated that 4 billion people lack access (318). There is a need to plan, develop, and implement effective FSM chains in areas with on-site sanitation facilities.

The results of FSM case studies emphasize context specific analysis is key to developing an effective, sustainable FSM program (78,169). Typically, sanitation services are fragmented, as interventions focus on sanitation technology provision at the household level, but neglect emptying, transport, and treatment needs (316). Data such as the volume of fecal waste generated and filling rates will need to be estimated in order to plan adequate transport, treatment and disposal systems (169). National governments may need to partner with the private sector to develop or refine the FSM system to determine appropriate tariffs, locations of FSTP sites, and hours of operation (76). In areas where hygienic services are non-existent, social franchising could be used to develop small emptying businesses (94).

### **Septic systems**

Septic systems are widely used across developed and developing settings and in both urban and rural areas. Properly functioning septic systems were found to provide effective on-site wastewater treatment. Studies reported substantial pathogen reduction in the septic tank, supporting previous comparisons of septic tank effluent with effluent of primary sedimentation tanks (188,319,320), and reductions between 99-100% after effluent dispersal through 60 cm of unsaturated soil.

Discharging septic tank effluent into canals or drains was found a common practice in some Southeast Asian cities. In Vientiane, the city government reportedly advocates discharging effluent in drains because the high water table in the city makes drain fields less effective (47). The review findings suggest that wastewater receives only partial treatment in the septic tank and that effluent is hazardous without secondary treatment. Discharging effluent directly into drains can be

considered “safe” disposal when drains lead to sewers (104) and, ultimately, wastewater treatment, but discharging effluent into drains leading to the environment can introduce high levels of bacteria, viruses, and helminth eggs into soils, groundwater, or receiving surface waters, which may be used for food production, drinking water, recreation, or other purposes.

A few studies in the review reported high prevalence of septic systems in unsuitable soils and high probabilities of system failure as a result. In the US, where nearly a quarter of the population uses septic systems, only a third of the land has soil suitable for septic system drain fields, and it is unknown how many systems are poorly sited (117). Similarly, approximately a third the population of Ireland uses septic tanks, and 39% of the land has high and very high susceptibility to groundwater contamination by on-site wastewater treatment, as determined by the EPA Risk-Based Methodology to Assist in the Regulation of Domestic Wastewater Treatment Systems (167). Installation of septic systems in poor sites or soils increases risk of groundwater contamination, and prevalence of septic systems on poor soils is largely unknown.

Groundwater contamination by septic systems is a major public health concern for areas where groundwater serves as a source for the drinking water supply. Domestic wells in areas with a high density of septic systems are more likely to pump septic system leachate (321), and well water contaminated by septic systems is the most common cause of waterborne disease outbreaks in the US (130). Septic systems and sewage were the most frequently reported sources of microbial groundwater contamination in a recent systematic review (17); over half of the studies on US groundwater contamination reported that septic systems and municipal sewage were potential contamination sources. Among US state water quality agencies, septic systems are the second-most frequently cited concern for groundwater contamination (322).

Septic systems were reported causes of groundwater and surface water contamination in several included studies, although most of these studies did not use statistical methods to analyze a correlation between septic systems and contamination. Failing systems were shown to introduce higher groundwater contamination, as were wet and rainy conditions, consistent with previous research (323). Few studies researched groundwater contamination by septic systems in developing countries, and the vulnerability to groundwater and surface water in these settings is not well-characterized in literature.

Different regulatory structures are available for monitoring installation and maintenance of septic systems. Septic system regulations are established at the state level in the US and are largely prescriptive, with varying standards for setback distance, maximum land slope, trench width, and distance between seasonally high water tables and the bottom of the drain field (117,324). An alternative regulatory approach is the performance-based approach, which evaluates use of alternative treatment systems in areas that are not suitable for conventional systems (325). Performance-based regulations take public and environmental health into account for system evaluation and incorporate education, monitoring, and enforcement into the evaluation framework (325). Using performance-based or hybrid prescriptive- and performance-based regulatory frameworks connects septic system users with alternative solutions specific to their local conditions, information, and incentives to maintain their systems.

Maintenance and desludging of septic systems were generally found to be infrequent across developed and developing settings. A few studies reported median desludging intervals of 4-8 years, but the range was wide, and many households either could not remember the last time their system was desludged or had never desludged the tank. Desludging knowledge and awareness as well as availability and cost of desludging services were common limiting factors but were highly regional and not generalizable.

Only a few countries have established regulations for septic tank desludging. Until 2008, Malaysia had a mandate for desludging septic systems at regular intervals; half of the country's households with septic tanks had scheduled desludging under federal law (163). After the mandate was lifted, demand for desludging services dropped and many private service operators went out of business (93). Under the Philippines' Clean Water Act and National Sewerage and Septage Management Programme (NSSMP), septage management is regulated, and some cities have enacted mandates for regular desludging (163). NSSMP will contribute up to 40% of costs to implement sewerage projects in local cities and municipalities (169). In San Fernando City, Philippines, trained plumbers were organized to survey septic system performance and survey users on sanitation conditions and interest in participating in the city's fecal sludge management program (169). Dedicated oversight can also ensure proper service delivery. For example, in Maharashtra, India, urban local bodies (ULBs) are tasked with septic system maintenance, and while several institutions oversee ULBs, no one institution monitors ULB service provision (163).

### **Sewerage**

Sewerage as a sanitation technology has been shown to reduce diarrheal incidence by 30% according to a recent systematic review (326). Often, the high capital cost of sewers is cited as the major concern as a sanitation solution. However, as the reviewed studies show, sewer connections alone are not sufficient to ensure adequate separation of humans from fecal waste. If wastewater is simply contained and transported away from one household, yet dumped in another household's vicinity, then the public health gains may be null across all households.

From the reviewed studies, it is uncertain what the extent and frequency of misconnections, exfiltration, and CSOs are on a global scale. Only a few studies were retrieved on the degree of misconnections and exfiltration. Most of the studies were from developed countries, therefore estimates within low and lower-middle countries are unknown. However, some of the reviewed studies demonstrated that large volumes of untreated wastewater are discharged to receiving waters through illicit connections to stormwater drains. Since combined sewers are only used in certain countries, it is not surprising that most reviewed studies were from developed countries.

Misconnections may result in untreated sewage being discharged in surface water that may have direct health implications. The investigation of a recent outbreak of *Cryptosporidium* in Sweden identified misconnections or sewage overflow as potential sources of contaminated wastewater that was discharged into receiving waters used for drinking water (327). Visual inspection, dye testing, smoke testing, distributed temperature sensing, infrared thermography, and water quality monitoring are methods that have been developed to detect misconnections and illegal connections

(328). However some of these methods may require highly trained personnel or advanced equipment that may not be appropriate for all settings.

Only a few of the studies on CSOs and SSOs quantified the frequency of events, however the results demonstrated overflows usually result in a high load of viruses, bacteria, and protozoa on receiving waters. A recent study on health outcomes and CSOs found an association between ER visits for gastrointestinal illness and heavy rainfall in areas with CSO outfalls to a water source used for drinking water (329). Different interventions such as storage, sewer rehabilitation, sewer separation, inflow reduction, disinfection, and constructed wetlands have been implemented in various areas to reduce the impact of CSOs on receiving waters (259,330–332).

The results of reviewed studies indicate high percentages of wastewater are discharged untreated into surface water bodies and leached into the ground. This Although Malik et al. reported on the percentage of wastewater treated in 107 countries, they found little data available on the level of wastewater treatment by country (271). Similarly, this review found only some reports and studies contained descriptions of treatment processes and usually only for developed countries at the national level and for a handful of developing countries but only at the city or regional level. Therefore it is difficult to determine on a global scale how much wastewater is treated using primary, secondary, and tertiary treatment. Even in cases where wastewater is treated, the results of reviewed studies reveal treatment facilities do not always operate effectively. Existing WWTP were often reported to be operating over or under their design capacity, which can negatively impact treatment processes.

In reviewing the effectiveness of different treatment processes, most efficiencies are reported in log reductions or percent removals. Log-reductions or percent removals do not reflect the final concentration of pathogens present in the effluent. In cases where waste streams contain high concentrations of pathogens, even 2-4 log reductions may not result in a final effluent that would qualify as a “safe” return of human excreta. A 2007 study estimated the raw wastewater concentration threshold for various pathogens that would result in an acceptable limit of 1 in 10,000 annual risk of infection (37). Assuming a 5 log reduction in pathogens based on conventional wastewater treatment methods, the study calculated raw concentrations of pathogens in wastewater would need to be between 0.022 to 191 organisms/L to meet the acceptable risk limit. As this review has shown, most raw wastewater concentrations are well above this range (37). Pathogens with a low infectious dose, that are resistant to treatment, and have the ability to persist in the environment pose unique challenges to wastewater treatment, and merit particular consideration in public health implications for downstream users.

The findings of this review have limitations. While the search for literature was strategic and extensive, it was not conducted systematically, and relevant articles from peer-reviewed and grey literature may not have been included in the review. The literature gaps identified in the review are indicative of readily accessible literature, but cannot fully characterize available literature. Only articles published in English and a handful of French studies were included in the review, therefore there may be additional relevant studies published in other languages. Additionally, assessment of study quality was not possible in the review timeframe, so study findings reported in the review are

subject to varying degrees of bias. Few studies described household sanitation practices, fecal sludge management, and wastewater treatment, and study results may not be generalizable to other settings. Furthermore, many of these studies were cross-sectional or observational and gathered data during one time period, so results may not fully represent current waste management in the described areas. For these reasons, interpretations of individual study results were limited within the review analysis, and the review conclusions drew more heavily from emergent findings across multiple studies.

## 7. Conclusions

- Latrines and septic systems are infrequently emptied across developed and developing areas. Full latrines are “flooded out” in some regions, releasing excreta directly into the environment. In other areas pits are buried once they become full which limits human exposure to excreta. Unlined pits, damaged latrines, and latrines in flood-prone areas pose contamination risks for groundwater. Methods for pit latrine emptying vary by setting, although manual emptying is a common practice in many regions.
- Contamination of groundwater by latrines and septic systems has been reported in many settings. Areas with a high density of latrines and/or septic systems have a higher probability of groundwater contamination, although vulnerability to contamination is dependent on a number of geological and climatic factors, such as soil type and rainfall.
- While many studies reported volumes of fecal sludge collected and transported in major cities, the disposal practices were underreported. Some cities reportedly dispose of sludge in the ocean, and others have no designated disposal sites.
- There are few estimates on frequency of sewerage misconnections, extent of exfiltration, and frequency of combined sewer overflows.
- The percentage of wastewater receiving treatment is available at the country level, but the amount of wastewater receiving primary, secondary, and tertiary treatment is largely unknown. The efficiency of wastewater treatment processes often focus on log reductions rather than final pathogen concentration, therefore treated effluent discharged to water bodies may still be an ‘unsafe’ return of excreta into the environment.

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## 9. Appendix A. Additional Tables

**Table 1. Septic tank coverage and failure rates in US States.** Adapted from (117). Sources: (333,334).

State	Septic tank or cesspool use (%)	Estimated system failure rate (%)	Failure definition
Alabama	43.6	20	Not given
Arizona	17.0	0.5	Surfacing, backup, surface or groundwater
California	9.8	1-4	Surfacing, backup, surface or groundwater
Florida	25.6	1-2	Surfacing, backup, surface or groundwater
Georgia	36.8	1.7	Public hazard
Hawaii	18.7	15-35	Improper construction, overflow
Idaho	34.6	20	Backup, surface or groundwater contamination
Kansas	17.9	10-15	Surfacing, nuisance conditions (for installations)
Louisiana	25.8	50	Not given
Maryland	18.1	1	Surfacing, surface or groundwater contamination
Massachusetts	26.7	25	Public health
Minnesota	25.3	50-70	Cesspool, surfacing, inadequate soil layer, leaking
Missouri	24.2	30-50	Backup, surface or groundwater contamination
Nebraska	17.8	40	Nonconforming system, water quality
New Hampshire	49.0	<5	Surfacing, backup
New Mexico	25.5	20	Surfacing
New York	20.2	4	Backup, surface or groundwater contamination
North Carolina	48.5	15-20	Not given
North Dakota	24.1	28	Backup, surfacing
Ohio	21.5	25-30	Backup, surfacing
Oklahoma	26.1	5-10	Backup, surfacing, discharge off property
Rhode Island	28.6	25	Not given
South Carolina	40.6	6-7	Backup, surface or groundwater contamination
Texas	18.1	10-15	Surfacing, surface or groundwater contamination
Utah	10.9	0.5	Surfacing, backup, exceed discharge standards
Washington	31.0	33	Public health hazard
West Virginia	40.8	60	Backup, surface or groundwater contamination
Wyoming	24.1	0.4	Backup, surfacing, groundwater contamination

**Table 2. Common non-conventional septic tank designs in developing countries.** Adapted from (93).

Country	Septic tank descriptions	Reference
Bangladesh	Multi-chamber tanks that have outflows connected to available drains.	(93)
Burkina Faso	A double chamber system made of concrete, the septic tank receives wastewater from the household. After decantation of the suspended solids in the second chamber, the effluent is dispersed by infiltration from a sump.	(93)
Cambodia	Most of the septic tanks were built under the French rule during the 50s. Sealed at the bottom to prevent infiltration to the environment, they are composed of two chambers with average volume from 2 to 3 cubic meters.	(93)
Ethiopia	An underground masonry wall or reinforced concrete tank having a compartment, with its effluent discharged to a soak away pit	(93)
India	Septic tanks are mostly single chambered units with variable sizes, depending on space availability, family size and affordability factors. A large number of single chamber septic tanks in urban poor settlements are deliberately designed with the mouths open to drain out excess water into the environment. In Jaipur, in addition to the commonly used single chamber septic tanks, the other widely used septic tank equivalents consist of “off-the-shelf” cylindrical concrete frames, bottom sealed, with holes on the sides to allow percolation. Households in Madurai generally prefer double-chambered septic tanks.	(93)
Kenya	Septic tanks refer to waterproof chambers (usually double rectangular) installed below ground to receive sewage. Septic tanks separate solid components (sludge) and liquid components. After separation, the liquid components leave the septic tank and are filtered through soakage pits or drainage fields and discharged to the soil.	(93)
Malaysia	The general capacity of a septic tank is designed based on a per capita wastewater generation rate of 225 liters per day (consisting of toilet waste and sullage) and a household size of 5 persons per residential premise (household survey results shown that this is generally true for the 3 cities studied). The minimum volumetric capacity of a septic tank should not be less than 2cu.m and consists of at least 2 compartments to allow for effective settlement of solid and retention of floatables	(93)
Nepal	Septic tanks are composed of one or two more chambers, or are concrete ring tanks. Septic tank walls are made of brick cement mortar, brick mud mortar, or reinforced cement concrete walls, and the inner walls of some tanks are not plastered with cement.	(13)
Nigeria	Rectangular single chambers cited below ground level, which receives both excreta and flush water from the toilets before the effluent is discharged into a soak away pit.	(93)
Senegal	Underground tank for the preliminary treatment of domestic wastewater, generally rectangular in shape, compartmentalized into two or three chambers, depending on the amount of water to be treated.	(93)
Thailand	Septic tanks are one or two chambered concrete ring tanks or commercial tanks. Concrete ring tanks are not sealed and have perforated walls to allow liquid seepage into the soil.	(128)
Tuvalu	Two chamber tanks that buried and are not sealed	(129)
Vietnam	Septic tanks are usually two or three chamber systems made from bricks, or reinforced concrete. The first, receiving chamber, often is built with largest portion of the tank volume, giving space for solids accumulation and anaerobic digestion. Total volume of the household septic tank, depending on available space and financial availability, often ranges from 1.5 to 5 m <sup>3</sup>	(93)

**Table 3. Bacterial travel in soil.** Adapted from (52,148).

Source	Medium	Organisms	Distance traveled (m)	Travel time (d)	Reference
<b>Pit latrines</b>					
Borehole latrine intersecting GW	Medium sand	-	21	-	(335) <sup>a</sup>
Borehole latrine intersecting GW	Medium sand	-	3	-	(336) <sup>a</sup>
Pit latrine dug to weathered and fractured bedrock	Fractured rock	-	25+	-	(337) <sup>a</sup>
Pit latrine intersecting GW	Fine and coarse sand	<i>Bacillus coli</i>	24.4	-	(338) <sup>a,b</sup>
Pit latrine intersecting GW	Sand and sandy clay	<i>Bacillus coli</i>	10.7	56	(339) <sup>a,b</sup>
Pit latrine intersecting GW	Fine and medium sand	<i>Bacillus coli</i>	3.1	-	(340) <sup>a,b</sup>
<b>Septic tanks</b>					
STE with shallow water table	Silty sand	-	3	-	(341) <sup>a</sup>
STE with imported fill material	Fine sand	-	16	-	(341) <sup>a</sup>
STE, rapid flow through macropores	Silty clay loam	-	15+	-	(342) <sup>a</sup>
STE in mountainous terrain	Fractured rock	-	28+	-	(202) <sup>a</sup>
Septic tank drain field 0.15 m above GW	Sandy clay	-	15	-	(343) <sup>a</sup>
Septic tank tile effluent	Fine loamy soil	Total coliforms	6.1	-	(190) <sup>b</sup>
Septic tank tile effluent	Fine loamy soil	Fecal coliforms	13.5	-	(190) <sup>b</sup>
Inoculated effluent in tile line	Silty clay loam	<i>E. coli</i>	20	0.2	(144) <sup>b</sup>
Septic tank drain field submerged in perched water table	Silty clay loam	-	15+	-	(342) <sup>a</sup>
<b>Infiltration beds</b>					
Sewage trenches intersecting GW	Fine sand	<i>Bacillus coli</i>	19.8	189	(344) <sup>b</sup>
Sewage trenches intersecting GW	-	Coliforms	70.7	-	(345) <sup>b</sup>
Primary sewage in infiltration basins	Silty sand and gravel	Fecal streptococci	183	-	(346) <sup>b</sup>
Primary and treated sewage in infiltration basins	Fine sandy loam	Coliforms	0.6-4	-	(197) <sup>b</sup>
Secondary sewage effluent in infiltration beds	Fine loamy sand	Fecal coliforms and fecal streptococci	9	-	(201) <sup>b</sup>
Secondary sewage in infiltration basins	Fine loamy sand to gravel	Fecal coliforms	9.1	-	(347) <sup>b</sup>

Secondary sewage in infiltration basins	Sandy gravels	Coliforms	0.9	-	(348) <sup>b</sup>
Tertiary treated water in infiltration beds	Fine to medium sand	Coliforms	6.1	-	(349) <sup>b</sup>
Tertiary treated wastewater in percolation beds	Fine to coarse sand aquifer	Coliforms	830	1-1.25	(350) <sup>b</sup>
Tertiary treated wastewater in percolation beds	Coarse gravels	Fecal coliforms and fecal streptococci	457.2	15	(351) <sup>b</sup>
Canal water in infiltration basins	Sand dunes	<i>Escherichia coli</i>	3.1	-	(352) <sup>b</sup>
<b>Subsurface injection</b>					
Subsurface injection	-	Enterococci	15	-	(353) <sup>b</sup>
Diluted primary sewage injected subsurface	Aquifer	Coliforms	30	1.4	(354) <sup>b</sup>
Primary sewage injected subsurface	Sand and pea gravel aquifer	Coliforms	30.5	1.5	(355) <sup>b</sup>
Secondary sewage injected subsurface	Fine to coarse sand aquifer	Fecal coliforms	30.5	-	(356) <sup>b</sup>
Inoculated water and diluted sewage injected subsurface	Crystalline bedrock	<i>Bacillus stearothermophilis</i>	28.7	-	(202) <sup>b</sup>
Injection of tracer organisms at land disposal site	Alluvial gravels	-	920	-	(357) <sup>a</sup>
<b>Other</b>					
Induced hydraulic gradient with sewage infiltration	Fine sand	-	122	-	(358) <sup>a</sup>
Artificial recharge with treated sewage direct to GW	Sand	-	30-68	-	(197) <sup>a</sup>
Simulated leaking sewer pipe	Alluvial gravels	-	125	-	(359) <sup>a</sup>

<sup>a</sup>As cited in Lewis et al. (1982)

<sup>b</sup>As cited in Crane and Moore (1984)

**Table 4. Estimated pollutant discharge per capita (PDC) for on-site wastewater treatment plants (WWTPs) in Bangkok, Thailand.** Adapted from (170).

	<b>Fecal coliforms (MPN person<sup>-1</sup> day<sup>-1</sup>)</b>	<b>Total coliforms (MPN person<sup>-1</sup> day<sup>-1</sup>)</b>
PDC of black water	$6.5 \times 10^{10}$	$3.2 \times 10^{11}$
PDC of on-site WWTP effluent	$1.2 \times 10^8$	$6.0 \times 10^8$
PDC of on-site WWTP seepage	$6.4 \times 10^{10}$	$3.2 \times 10^{11}$
PDC of on-site WWTP septage	$3.6 \times 10^8$	$1.8 \times 10^9$
PDC of on-site WWTP seepage flowing into ambient water	$2.5 \times 10^7$	$1.2 \times 10^8$

**Table 5. Anaerobic digestion process efficiencies in removing pathogens.** Adapted from (225).

Pathogen	Sludge type	Digestion	Digestion process	Temp (C) <sup>a</sup>	Time (days) <sup>b</sup>	Log reduction or T <sub>90</sub> (d) <sup>a</sup>	Country
Total coliforms	Sewage sludge	M	Continuous/ semi-continuous	35	15	2-3	Spain
	Cattle dung and sewage sludge	M	Batch	35	30	4.53	Nepal
	Sewage sludge	M	Unknown	-	-	1.6	US
	Sewage sludge	M	Unknown	-	-	2.2	US
	Sewage sludge	M	Unknown	36	20	0.3	Canada
	Human excreta and food waste	P		17	-	2.7	Ethiopia
	Human excreta and food waste	P		17	-	1.4	Ethiopia
Fecal coliforms	Cattle dung and sewage sludge	P	Batch	10-20	35	2.1	Nepal
	Sewage sludge	M	Batch	37	21	3.0	North America
	Sewage sludge	M	Batch	37	30-35	3.0	North America
	Cattle dung and sewage sludge	M	Batch	35	30	5.16	Nepal
	Sewage sludge	M	Unknown	-	-	1.3	US
	Sewage sludge	M	Unknown	-	-	2.4	US
	Sewage sludge	M	Unknown	36	20	1.56	Canada
	Human excreta and food waste	P	Continuous	17	-	2.8	Ethiopia
	Human excreta and food waste	P	Continuous	17	-	0.7	Ethiopia
	Cattle dung and sewage sludge	P	Batch	10-20	35	2.5	Nepal
<i>E. coli</i>	Liquid sewage sludge	M	Batch	35	15-20	0.5-2.0	UK
	Sewage sludge	M	Semi-continuous	35	21 (12)	1.48-1.68	UK
	Sewage sludge	M	Batch	37	21	1-2	North America
	Sewage sludge	M	Batch	37	21	2.0	North America
<i>Salmonella spp.</i>	Liquid sewage sludge	M	Batch	35	15-20	-0.5-2.5	UK

	Sewage sludge	M	Semi-continuous	35	15	2.23	UK
	Sewage sludge	M	Semi-continuous	35	22 (12)	1.91	UK
	Sewage sludge	M	Continuous/ Semi-continuous	35	15	0.2	Spain
	Sewage sludge	M	Unknown	-	-	1.6	US
<i>Enterococcus spp.</i>	Sewage sludge	M	Unknown	-	-	0.7	US
	Sewage sludge	M	Unknown	-	-	1.6	US
	Sewage sludge	M	Unknown	36	20	No change	Canada
<i>Listeria monocytogenes</i>	Sewage sludge	M	Semi-continuous	35	21 (12)	2.23	UK
<i>Campylobacter jejuni</i>	Sewage sludge	M	Semi-continuous	35	21 (12)	-1.0	UK
<i>Vibrio cholera</i>	Human night soil	M	Semi-continuous	23-27	20 (10)	6	India
	Human night soil	M	Semi-continuous	23-27	30 (20)	3.5	India
<i>Clostridium perfringens</i>	Sewage sludge	M	Unknown	36	20	No change	Canada
Enteroviruses	Sewage sludge	M	Unknown	-	-	1.6	US
	Sewage sludge	M	Unknown	-	-	1.1	US
	Sewage sludge	M	Semi-continuous	34	2	2.0	UK
	Sewage sludge	M	Unknown		400	0-2.0	US
Poliovirus	Sewage sludge	M	Semi-continuous	35	21 (12)	6.2	UK
Somatic coliphages	Sewage sludge	M	Unknown	36	20	0.09	Canada
<i>Cryptosporidium</i>	Sewage sludge	M	Unknown	36	20	0.30	Canada
<i>Giardia</i>	Sewage sludge	M	Unknown	36	20	No change	Canada
	Sewage sludge	M	Unknown		400	0 to -2.0	US
<i>Ascaris suum ova</i>	Sewage sludge	M	Semi-continuous	34	26	No change	UK

M- mesophilic, P-psychrophilic

<sup>a</sup>In some cases data have been summarized, calculated, or approximated from original paper.

<sup>b</sup>Time taken for log reduction.

**Table 6. Reported treatment efficiencies for waste stabilization ponds in literature.**

Country	WSP system	RT (days)	Treatment Facilities (N)	FC % removal Fecal coliforms Inlet Outlet	<i>Campylobacter</i> (/100 mL)	<i>Salmonella</i> (/100 mL)	Enteroviruses (/10 mL)	Rotaviruses (/10 mL)	Helminths (eggs/L)	<i>Giardia</i> (oocysts/L)	Ref
Brazil	FP	NA	65	97.5% 5.3 x 10 <sup>7</sup> (MPN/100 mL) 1.2 x 10 <sup>6</sup> (MPN/100 mL)							(290)
Brazil	AP + FP		40	99.4% 2.0 x 10 <sup>8</sup> (MPN/100 mL) 4.3 x 10 <sup>5</sup> (MPN/100 mL)							(290)
Brazil	AP + FP	6.8 5.5		99.4% 5 x 10 <sup>7</sup> (CFU/100 mL) 3 x 10 <sup>5</sup> (CFU/100 mL)					99.88% 804 1 99.33% 1489 10		(360)
Brazil	AP + FP +3 MP	6.8 5.5 5.5-5.8 4.0 3.2 3.2-3.4		99.9999% 5 x 10 <sup>7</sup> (CFU/100 mL) 30 (CFU/100 mL)					100% 804 0 99.73% 1489 4		(360)
Brazil	AP+ FP	1 5		96% 2 x 10 <sup>7</sup> (CFU/100 mL) 8 x 10 <sup>5</sup> (CFU/100 mL)	99.97% 70 0.2	99.5% 20 0.1	90% 1 x 10 <sup>4</sup> 1 x 10 <sup>3</sup>	91.25% 800 70			(361)
Brazil	AP + FP + 3 MP	1 5 5		99.97% 2 x 10 <sup>7</sup> (CFU/100 mL)	100% 70 0	0% 20 0	99.91% 1 x 10 <sup>4</sup> 9	99.63% 800 3			(361)

				$7 \times 10^3$ (CFU/100 mL)			
Kenya	11 SP	14-133				100%	(362)
						13-73	
						0	
						100%	
						213-6213	
						0	
Morocco	SP	16				100%	(363)
						0	
Iran	SP	NR	2			100%	100%
						37.99	9.11
						0	0
						100%	100%
						29.98	7.6
						0	0

AP – anaerobic pond  
 FP – facultative pond  
 MP- maturation pond  
 SP- stabilization pond

**Table 7. *Cryptosporidium parvum* concentration in wastewater samples.** Adapted from (300).

<b>Sample</b>	<b>N</b>	<b>% positive (total oocysts)</b>	<b>% positive (infectious oocysts)</b>	<b>Range (infectious oocysts) oocysts/100 mL</b>
Influent	18	78	33	36.8-5,065
Secondary effluent	18	83	39	<2.5-106
Postfiltration	17	71	35	<2.2-19
Final disinfected effluent	15	67	40	<1-27

**Table 8. Pathogen removal efficiency in wastewater effluent from various steps in wastewater treatment process.** Adapted from (288).

Pathogen	Untreated wastewater	Post-clarification	Post-filtration	Post-chlorination	Storage tank
<b>TC</b>					
% positive	100	100	100	18	18
Geometric mean (CFU/100 mL)	(1.3 x 10 <sup>7</sup> )	(4.1 x 10 <sup>5</sup> )	(1.4 x 10 <sup>5</sup> )	(0.9)	(0.60)
% reduction		98.3%	69.3%	99.99%	75.4%
Log removal		1.75	0.51	4.23	0.61
<b>FC</b>					
% positive	100	100	100	9	9
Geometric mean (CFU/100 mL)	(1.6 x 10 <sup>6</sup> )	(6.0 x 10 <sup>4</sup> )	(4.6 x 10 <sup>4</sup> )	(0.30)	(0.20)
% reduction		99.1%	10.5%	99.998%	56.8%
Log removal		2.06	0.05	4.95	0.36
<b>Phage</b>					
% positive	100	90	92	50	25
Geometric mean (PFU/100 mL)	(7.1 x 10 <sup>3</sup> )	(2.7 x 10 <sup>2</sup> )	(3.1 x 10)	(0.90)	(0.30)
% reduction		82.1%	99.98%	90.5%	90.3%
Log removal		0.75	3.81	1.03	1.03
<b>Enteroviruses</b>					
% positive	100	58	50	25	8
Geometric mean (PFU/100 mL)	(4.2 x 10 <sup>2</sup> )	(5.3)	(1.5)	(0.09)	(0.01)
% reduction		98.0%	84.0%	96.5%	90.91%
Log removal		1.71	0.81	1.45	1.04
<b>Giardia</b>					
% positive		83	75	42	25
Geometric mean (cysts/100 mL)	100 (3.9 x 10 <sup>3</sup> )	(8.8 x 10)	(2.6)	(0.05)	(0.30)
% reduction		93.0%	99.0%	78.0%	49.5%
Log removal		1.19	2.00	0.65	0.30
<b>Cryptosporidium</b>					
% positive					
Geometric mean (oocysts/100 mL)	67 (3.7 x 10 <sup>2</sup> )	42 (3.5 x 10)	42 (2.9)	25 (1.0)	17 (0.30)
% reduction		92.8%	97.9%	61.1%	8.5%
Log removal		1.14	1.68	0.41	0.04
<b>Helminths</b>					
% positive	33	0	0	0	0
Geometric mean (ova/L)	(4.3)	(-)	(-)	(-)	(-)
% reduction		>75%			
Log removal					

**Table 9. Pathogen removal efficiency in fecal sludge from various steps in wastewater treatment process.**  
Adapted from (288).

Pathogen	Raw sludge	Thickened sludge	Digested sludge	Dewatered sludge
<b>TC</b>				
% positive	100	100	100	100
Geometric mean (CFU/100 mL)	$2.6 \times 10^7$	$1.7 \times 10^7$	$2.3 \times 10^6$	$5.4 \times 10^5$
% reduction		<1%	92%	11%
Log removal		<0.01	1.07	0.05
<b>FC</b>				
% positive	100	100	100	100
Geometric mean (CFU/100 mL)	$9.4 \times 10^6$	$6.1 \times 10^6$	$1.1 \times 10^6$	$2.4 \times 10^5$
% reduction		<1%	92%	2.4%
Log removal		<0.01	1.11	0.12
<b>Phage</b>				
% positive	100	100	100	100
Geometric mean (PFU/100 mL)	$4.2 \times 10^5$	$3.4 \times 10^5$	$7.4 \times 10^4$	$3.1 \times 10^4$
% reduction		24%	84%	<1%
Log removal		0.12	0.80	<1
<b>Enteroviruses</b>				
% positive	100	100	100	100
Geometric mean (PFU/100 mL)	37	14	8	11
% reduction		57%	48%	<1%
Log removal		0.36	0.30	<1
<b>Giardia</b>				
% positive	50	0	25	25
Geometric mean (cysts/g)	$2.8 \times 10^2$	NA	150	35
% reduction		>89%	<1%	77%
Log removal		>0.96	<1	0.63
<b>Cryptosporidium</b>				
% positive	25	25	50	0
Geometric mean (oocysts/g)	$3.2 \times 10^3$	410	61	NA
% reduction		87%	58%	>90%
Log removal		0.89	0.38	>1.00
<b>Helminths</b>				
% positive				
Geometric mean	NA	NA	NA	NA
% reduction				
Log removal				

Table 10. Sludge treatment methods used in EU member nations. Adapted from (307).

Country	Stabilization				Conditioning				Dewatering				Others				
	Aerobic	Anaerobic	Lime	Compost-ing	Lime	Other inorganics	Poly-mers	Thermal	Drying beds	Filter press	Centri-fuges	Belt filter press	Thermal drying	Solar drying	Pasteuri-zation	Long term storage	Cold ferment-ation
Austria	XX	XX	X	XX		X	XX			XX	XX	XX	XX		XX		
Belgium (Flanders)	XX	XX	XX	X									XXX				XX
Belgium (Wallonia)	XX	XX		X	XX		XX			XX	XX	XX	XX				
Denmark	XX	XX	XX	XX									XX		XX		
Finland	XX	XXX	X	XXX													
France	XX	XX	XX	XX	XX					XX	XX		XX	XX			
Germany		XX	XX	X				XX					XXX				
Greece	XX	XX	XX	X			XX		XX		XX	XX	XX	XX			
Ireland	XX	XX	XX	X							XX	XX	XX			XX	
Italy	XX	XXX	XX	XX	XX	XX		XX	XX	XX	XX	XX	X		X		
Luxembourg		XX		XX	XX	XX	XX									XX	
Netherlands	XX	XX		XX									XX				
Portugal	XX	XX							XX	XX	XX	XX	XX				
Spain	XX	XXX	XX	X									XX			XX	
Sweden	XX	XX	XX	XX		XX		XX	XX		XXX	XX	XX				
UK	XX	XXX	XX	XX							XX	XX	XX			XX	
Bulgaria	XX	XX	XX	X						XX						XX	
Cyprus	XX	XX		X						XX	XX					XX	
Czech Republic	XX	XXX		XXX													
Estonia		XX		XXX													
Hungary		XX		XXX							XX	XX				XX	
Latvia		XX		X													X
Lithuania	XX		XX	XX												XXX	
Malta																	
Poland	XXX	XX	XX	X					XX	XX	XX	XXX				XX	
Romania				XX					XX								
Slovakia	XX	XXX	X	XXX													
Slovenia	XX	X		X						XX	X	XX	XX				

X, rare method  
 XX, common use  
 XXX, most common use